FOREST GROVE OREGON

A place where families and businesses thrive.

J3150
FC15

Building Permit Application

City of Forest Grove

Permit Number: 311-23-000206-DWL

TYPE OF WORK	
☑ New construction	☐ Demolition
☐ Addition/alteration/replacement	☐ Other:
CATEGORY OF CONSTRUCTION	
☑ 1- and 2-family dwelling	☐ Commercial/industrial
☐ Accessory building	☐ Multi-family
☐ Master builder	☐ Other:
JOB SITE INFORMATION AND LOCATION	
Job site address: 3730 Tiana St.	
City/State/ZIP: Forest Grove, OR 97116	
Suite/bldg./apt. no.:	Project name: Farmstead Crossing
Cross street/directions to job site:	
Subdivision: Farmstead Crossing	Lot no.: 15
Tax map/parcel no.:	
DESCRIPTION OF WORK	
New, single family residence	
☑ PROPERTY OWNER	☐ TENANT
Name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
☑ APPLICANT ☐ ENGINEER ☐ ARCHITECT/DESIGNER	
Business name: Stone Bridge Homes NW, LLC	
Contact name: Permit Tech	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CONTRACTOR	
Business name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email portlandpermits@stonebridgehomesnw.com
CCB lic.: 173318	City/Metro Bus lic:
Authorized signature: *Tiana Rudolf*	
Print name: Tiana Rudolf	Date: 3-27-2023

REQUIRED DATA: 1- AND 2-FAMILY DWELLING		
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.		
Valuation 209,857	$321,953.95	
Number of bedrooms: 3		
Number of bathrooms: 2		
Total number of floors: 1		
New dwelling area: 1,858		square feet
Garage/carport area: 567		square feet
Covered porch area: 244		square feet
Deck area:		square feet
Other structure area:		square feet
REQUIRED DATA: COMMERCIAL-USE CHECKLIST		
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.		
Valuation		
Existing building area:		square feet
New building area:		square feet
Number of stories:		
Type of construction:		
Occupancy groups:		
Existing:		
New:		

NOTICE

All contractors and subcontractors are required to be licensed with the Oregon Construction Contractors Board under ORS 701 and may be required to be licensed in the jurisdiction in which work is being performed. If the applicant is exempt from licensing, the following reasons apply:

BUILDING PERMIT FEES*	
Please refer to fee schedule	
Fees due upon application	
Amount received	
Date received	

This permit application expires if a permit is not obtained within 180 days after it has been accepted as complete

* Fee methodology set by Tri-County Building Industry Service Board

$37,866.17 paid
BUILDING

FOREST GROVE OREGON

A place where families and businesses thrive.

Plumbing Permit Application

City of Forest Grove

Permit Number: 311-23-000206-PLBL

TYPE OF WORK	
■ New construction	☐ Demolition
☐ Addition/alteration/replacement	☐ Other:
CATEGORY OF CONSTRUCTION	
■ 1- and 2-family dwelling	☐ Commercial/industrial
☐ Accessory building	☐ Multi-family
☐ Master builder	☐ Other:
JOB SITE INFORMATION AND LOCATION	
Job site address: 3730 Tiana St.	
City/State/ZIP: Forest Grove, OR 97116	
Suite/bldg./apt. no.:	Project name: Farmstead Crossing
Cross street/directions to job site:	
Subdivision: Farmstead Crossing	Lot no.: 15
Tax map/parcel no.:	
DESCRIPTION OF WORK	
New, single family residence	
■ **PROPERTY OWNER**	☐ **TENANT**
Name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: (503) 387-7577	Email: portlandpermits@stonebridgehomesnw.com
■ **APPLICANT**	☐ **CONTACT PERSON**
Business name: Stone Bridge Homes NW, LLC	
Contact name: Permit Tech	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: (503) 387-7577	Email: portlandpermits@stonebridgehomesnw.com
CONTRACTOR	
Business name: Edward Mullen Plumbing	
Address: S.E. River Rd.	
City/State/ZIP: Hillsboro, OR 97123	
Phone: (503) 640-0113	
City/Metro Bus Lic:	Email: jeremy@edwardmullenplumbing.com
CCB lic.: 92689	PB Lic. no.: 34-260PB

Authorized signature: *Tiana Rudolf*

Print name: Tiana Rudolf	Date: 3-27-2023

FEE* SCHEDULE			
For special information use checklist.			
Description	Qty.	Ea.	Total
New 1- 2-family dwellings (includes 100 ft. for each utility connection)			
SFR (1) bath		239.50	
SFR (2) bath	1	316.75	
SFR (3) bath		386.25	
Each additional bath/kitchen		41.72	
Fire sprinkler (_____ sq. ft.)		By sq ft	
Site utilities			
Catch basin or area drain		13.90	
Drywell, leach line, or trench drain		13.90	
Footing drain (each 100 ft.: _____)		46.35	
Manufactured home utilities			
Manholes		13.90	
Rain drain connector		13.90	
Sanitary sewer (each 100 ft.: _____)		46.35	
Storm sewer (each 100 ft.: _____)		46.35	
Water service (each 100 ft.: _____)		46.35	
Fixture or item			
Absorption valve		13.90	
Backflow preventer		13.90	
Backwater valve		13.90	
Clothes washer	1	13.90	
Dishwasher	1	13.90	
Drinking fountain		13.90	
Ejectors/sump		13.90	
Expansion tank		13.90	
Fixture/sewer cap		13.90	
Floor drain/floor sink/hub		13.90	
Garbage disposal	1	13.90	
Hose bib	2	13.90	
Ice maker		13.90	
Interceptor/grease trap		13.90	
Medical gas (value: $ _____)		By value	
Primer		13.90	
Roof drain (commercial)		13.90	
Sink/basin/lavatory	5	13.90	
Tub/shower/shower pan	2	13.90	
Urinal		13.90	
Water closet	2	13.90	
Water heater	1	13.90	
Other:		13.90	
Other:			
		Subtotal	
		Minimum permit fee	$ 27.30
		Plan review (___% of permit fee)	
		State surcharge (12% of permit fee)	
		TOTAL PERMIT FEE	

This permit application expires if a permit i s not obtained within 180 days after it has been accepted as complete.

*Fee methodology set by Tri-County Building Industry Service Board

Phone: 503-992-3229 Fax: 503-992-3202
1924 Council Street/P.O. Box 326, Forest Grove OR 97116

IVR Inspection Request Line: 888-299-2821

FOREST GROVE OREGON

A place where families and businesses thrive.

Mechanical Permit Application

City of Forest Grove

Permit No.: 311-23-000206-DWL

TYPE OF WORK	
☑ New construction	☐ Addition/alteration/replacement
☐ Demolition	☐ Other:

CATEGORY OF CONSTRUCTION		
☑ 1- and 2-family dwelling	☐ Commercial/industrial	☐ Accessory building
☐ Multi-family	☐ Master builder	☐ Other:

JOB SITE INFORMATION AND LOCATION

Job site address: 3730 Tiana St.

City/State/ZIP: Forest Grove, OR 97116

Suite/bldg./apt. no.: | Project name: Farmstead Crossing

Cross street/directions to job site:

Subdivision: Farmstead Crossing | Lot no.: 15

Tax map/parcel no.:

DESCRIPTION OF WORK

New, single family residence

☑ PROPERTY OWNER ☐ TENANT

Name: Stone Bridge Homes NW, LLC

Address: 4230 Galewood St. Suite #100

City/State/ZIP: Lake Oswego, OR 97035

Phone: 503-387-7577 | Email:

☑ APPLICANT ☐ CONTACT PERSON

Business name: Stone Bridge Homes NW, LLC

Contact name:

Address: 4230 Galewood St. Suite #100

City/State/ZIP: Lake Oswego, OR 97035

Phone: 503-387-7577 | Email: portlandpermits@stonebridgehomesnw.com

CONTRACTOR

Business name: Comfort Zone

Address: 1032 NW Corportate Dr.

City/State/Zip: Troutdale, OR 97060

Phone: 503-667-5595 | Email: portlandpermits@stonebridgehomesnw.com

CCB Lic No: 110091 | City/Metro BusLic No:

Authorized Signature: *Tiana Rudolf*

Print name: Tiana Rudolf | Date: 3-27-2023

This permit is issued under OAR 918-460-0030. This permit application expires if a permit is not obtained within 180 days after it has been accepted as complete.

COMMERCIAL FEE SCHEDULE- USE CHECKLIST

Mechanical permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all mechanical materials, equipment, labor, overhead. and profit.

Value: $

RESIDENTIAL EQUIPMENT/ SYSTEMS FEES

For special information use checklist.

Description	*Qty*	*Ea*	*Total*
Heating/Cooling			
Furnace	1	11.90	
Air Conditioning		11.90	
Heat Pump		8.95	
Duct Work		15.85	
Hydronic hot water system		15.85	
Residential boiler (radiator or Hydronic)		11.90	
Unit Heaters (fuel-type, not electric) in-wall, in-duct, suspended, etc.		11.90	
Flue/vent for any of the above		6.00	
Other		8.95	
Other fuel appliances			
Water heater	1	8.95	
Gas fireplace	1	8.95	
Flue vent for water heater or gas fireplace	2	6.00	
Log lighter		8.95	
Wood/pellet stove		8.95	
Wood fireplace/insert		8.95	
Chimney/liner/flue/vent		6.00	
Other:		8.95	
Environmental exhaust & ventilation			
Range hood/other kitchen equipment	1	8.95	
Clothes dryer exhaust	1	8.95	
Single-duct exhaust (bathrooms, toilet compartments, utility rooms	5	6.00	
Attic/crawl space fans		6.00	
Other		8.95	
Fuel piping $ 4.00 for first four; $1.05 for each additional			
Furnace, etc.	1	By Outlet #	
Gas heat pump		By Outlet #	
Wall/suspended/unit heater		By Outlet #	
Water heater	1	By Outlet #	
Fireplace	1	By Outlet #	
Range	1	By Outlet #	
Barbeque		By Outlet #	
Clothes dryer (gas)		By Outlet #	
Other		By Outlet #	
MECHANICAL PERMIT FEES*			
		Subtotal	
		Minimum Permit Fee	27.30
		Plan review(% of permit fee)	
		State surcharge (12% of permit fee)	
		TOTAL PERMIT FEE	

*Fee methodology set by Tri-County Building Industry Service Board

IVR Inspection request line: 1-888-299-2821

Phone: 503-992-3229 Fax: 503-992-3202
1924 Council Street/P.O. Box 326
Forest Grove OR 97116

FOREST GROVE OREGON

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Building Permit Application

City of Forest Grove

Permit Number: 311-23-000206-DWL

TYPE OF WORK	
☑ New construction	☐ Demolition
☐ Addition/alteration/replacement	☐ Other:
CATEGORY OF CONSTRUCTION	
☑ 1- and 2-family dwelling	☐ Commercial/industrial
☐ Accessory building	☐ Multi-family
☐ Master builder	☐ Other:
JOB SITE INFORMATION AND LOCATION	
Job site address: 3730 Tiana St.	
City/State/ZIP: Forest Grove, OR 97116	
Suite/bldg./apt. no.:	Project name: Farmstead Crossing
Cross street/directions to job site:	
Subdivision: Farmstead Crossing	Lot no.: 15
Tax map/parcel no.:	
DESCRIPTION OF WORK	
New, single family residence	
☑ PROPERTY OWNER	☐ TENANT
Name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
☑ APPLICANT ☐ ENGINEER ☐ ARCHITECT/DESIGNER	
Business name: Stone Bridge Homes NW, LLC	
Contact name: Permit Tech	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CONTRACTOR	
Business name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CCB lic.: 173318	City/Metro Bus lic:
Authorized signature: *Tiana Rudolf*	
Print name: Tiana Rudolf	Date: 3-27-2023

REQUIRED DATA: 1- AND 2-FAMILY DWELLING		
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.		
Valuation 209,857	$321,953.95	
Number of bedrooms: 3		
Number of bathrooms: 2		
Total number of floors: 1		
New dwelling area: 1,858		square feet
Garage/carport area: 567		square feet
Covered porch area: 244		square feet
Deck area:		square feet
Other structure area:		square feet
REQUIRED DATA: COMMERCIAL-USE CHECKLIST		
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.		
Valuation		
Existing building area:		square feet
New building area:		square feet
Number of stories:		
Type of construction:		
Occupancy groups:		
Existing:		
New:		

NOTICE

All contractors and subcontractors are required to be licensed with the Oregon Construction Contractors Board under ORS 701 and may be required to be licensed in the jurisdiction in which work is being performed. If the applicant is exempt from licensing, the following reasons apply:

BUILDING PERMIT FEES*	
Please refer to fee schedule	
Fees due upon application	
Amount received	
Date received	

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* Fee methodology set by Tri-County Building Industry Service Board

PLANNING DEVELOPMENT

1924

FOREST GROVE OREGON

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J3150
FC15

Building Permit Application

City of Forest Grove

Permit Number: 311-23-000206-DWL

TYPE OF WORK	
☑ New construction	☐ Demolition
☐ Addition/alteration/replacement	☐ Other:
CATEGORY OF CONSTRUCTION	
☑ 1- and 2-family dwelling	☐ Commercial/industrial
☐ Accessory building	☐ Multi-family
☐ Master builder	☐ Other:
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City/State/ZIP: Forest Grove, OR 97116	
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Subdivision: Farmstead Crossing	Lot no.: 15
Tax map/parcel no.:	
DESCRIPTION OF WORK	
New, single family residence	
☑ PROPERTY OWNER	☐ TENANT
Name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
☑ APPLICANT ☐ ENGINEER ☐ ARCHITECT/DESIGNER	
Business name: Stone Bridge Homes NW, LLC	
Contact name: Permit Tech	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CONTRACTOR	
Business name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CCB lic.: 173318	City/Metro Bus lic:
Authorized signature: Tiana Rudolf	
Print name: Tiana Rudolf	Date: 3-27-2023

REQUIRED DATA: 1- AND 2-FAMILY DWELLING		
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Valuation		
Existing building area:		square feet
New building area:		square feet
Number of stories:		
Type of construction:		
Occupancy groups:		
Existing:		
New:		
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BUILDING PERMIT FEES*	
Please refer to fee schedule	
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Date received	

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* Fee methodology set by Tri-County Building Industry Service Board

1924

ENGINEERING

FOREST GROVE OREGON

A place where families and businesses thrive.

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FC15

RECEIVED
4/3/23

Building Permit Application

City of Forest Grove

Permit Number: 311-23-000206-DWL

TYPE OF WORK	
☑ New construction	☐ Demolition
☐ Addition/alteration/replacement	☐ Other:
CATEGORY OF CONSTRUCTION	
☑ 1- and 2-family dwelling	☐ Commercial/industrial
☐ Accessory building	☐ Multi-family
☐ Master builder	☐ Other:
JOB SITE INFORMATION AND LOCATION	
Job site address: 3730 Tiana St.	
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Subdivision: Farmstead Crossing	Lot no.: 15
Tax map/parcel no.:	
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New, single family residence	
☑ PROPERTY OWNER	☐ TENANT
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Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
☑ APPLICANT ☐ ENGINEER ☐ ARCHITECT/DESIGNER	
Business name: Stone Bridge Homes NW, LLC	
Contact name: Permit Tech	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CONTRACTOR	
Business name: Stone Bridge Homes NW, LLC	
Address: 4230 Galewood St. Suite #100	
City/State/ZIP: Lake Oswego, OR 97035	
Phone: 503-387-7577	Email: portlandpermits@stonebridgehomesnw.com
CCB lic.: 173318	City/Metro Bus lic:
Authorized signature: *Tiana Rudolf*	
Print name: Tiana Rudolf	Date: 3-27-2023

REQUIRED DATA: 1- AND 2-FAMILY DWELLING		
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Covered porch area: 244		square feet
Deck area:		square feet
Other structure area:		square feet
REQUIRED DATA: COMMERCIAL-USE CHECKLIST		
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.		
Valuation		
Existing building area:		square feet
New building area:		square feet
Number of stories:		
Type of construction:		
Occupancy groups:		
Existing:		
New:		

NOTICE

All contractors and subcontractors are required to be licensed with the Oregon Construction Contractors Board under ORS 701 and may be required to be licensed in the jurisdiction in which work is being performed. If the applicant is exempt from licensing, the following reasons apply:

BUILDING PERMIT FEES*	
Please refer to fee schedule	
Fees due upon application	
Amount received	
Date received	

This permit application expires if a permit is not obtained within 180 days after it has been accepted as complete

* Fee methodology set by Tri-County Building Industry Service Board

1924 Co

LIGHT & POWER
CC 4/6/23

City of Forest Grove
1924 Council St
Forest Grove, OR 97116
503-992-3229
Fax: 503-992-3202

Building Permit

Residential 1 & 2 Fam Dwelling (New Only) Limited

Permit Number: 311-23-000206-DWL

IVR Number: 311095423292

Web Address: www.forestgrove-or.gov

Email Address: cd@forestgrove-or.gov

Permit Issued: May 22, 2023

Application Date: March 27, 2023

Project: FARMSTEAD CROSSING

TYPE OF WORK

Residential Specialty Code Edition: 2021

Category of Construction: Single Family Dwelling

Type of Work: New

Calculated Job Value: $321,953.95

Description of Work: NSFR- FARMSTEAD CROSSING - LOT 15 - PLAN 00 - 3 BED - 2 BATH - 1 FLOOR

JOB SITE INFORMATION

Worksite Address
3730 TIANA ST
FOREST GROVE OR 97116

Parcel
FARMSTEAD TEMPORARY

Owner: STONE BRIDGE HOMES NW LLC

Address: 4230 Galewood Street, Suite #100
LAKE OSWEGO, OR 97035

LICENSED PROFESSIONAL INFORMATION

Business Name	License	License Number	Phone
STONE BRIDGE HOMES NW LLC - Primary	CCB	173318	503-387-7577
THE MULLEN COMPANY	CCB	92689	503-640-0113
COMFORT ZONE HEATING AND AIR CONDITIONING INC	CCB	110091	503-667-5595

PENDING INSPECTIONS

Permits expire if work is not started within 180 Days of issuance or if work is suspended for 180 Days or longer depending on the issuing agency's policy.

Per R105.7 and R 106.3.1, a copy of the building permit and one set of approved construction documents shall be available for review at the work site.

All provisions of laws and ordinances governing this type of work will be complied with whether specified herein or not. Granting of a permit does not presume to give authority to violate or cancel the provisions of any other state or local law regulating construction or the performance of construction.

ATTENTION: Oregon law requires you to follow rules adopted by the Oregon Utility Notification Center. Those rules are set forth in OAR 952-001-0010 through OAR 952-001-0090. You may obtain copies of the rules by calling the Center at (503) 232-1987.

All persons or entities performing work under this permit are required to be licensed unless exempted by ORS 701.010 (Structural/Mechanical), ORS 479.540 (Electrical), and ORS 693.010-020 (Plumbing).

Inspection	Inspection Group	Inspection Status
1020 Zoning/Setbacks	1_2 Famdwell	Pending
1060 Driveway Approach	1_2 Famdwell	Pending
1065 Sidewalk	1_2 Famdwell	Pending
1110 Footing	1_2 Famdwell	Pending
1120 Foundation	1_2 Famdwell	Pending
1220 Underfloor Framing/Post and Beam	1_2 Famdwell	Pending
1260 Framing	1_2 Famdwell	Pending
1430 Insulation Wall	1_2 Famdwell	Pending
1520 Interior Shearwall	1_2 Famdwell	Pending
1530 Exterior Shearwall	1_2 Famdwell	Pending
1999 Final Building	1_2 Famdwell	Pending
2200 Underfloor Mechanical	1_2 Famdwell	Pending
2250 Gas Piping/Pressure Test	1_2 Famdwell	Pending
2300 Rough Mechanical	1_2 Famdwell	Pending
2999 Final Mechanical	1_2 Famdwell	Pending
3130 Footing/Foundation Drains	1_2 Famdwell	Pending
3170 Underfloor Plumbing	1_2 Famdwell	Pending
3200 Sanitary Sewer	1_2 Famdwell	Pending
3300 Water Service	1_2 Famdwell	Pending
3400 Storm Sewer	1_2 Famdwell	Pending
3410 Rain Drains	1_2 Famdwell	Pending
3500 Rough Plumbing	1_2 Famdwell	Pending
3999 Final Plumbing	1_2 Famdwell	Pending
6010 Preliminary Erosion Control	1_2 Famdwell	Pending
6030 Erosion Control	1_2 Famdwell	Pending
6050 Final Erosion Control	1_2 Famdwell	Pending
1996 Final Inspection - Planning	1_2 Famdwell	Pending

SCHEDULING INSPECTIONS

Various inspections are minimally required on each project and often dependent on the scope of work. Contact the issuing jurisdiction indicated on the permit to determine required inspections for this project.

Schedule or track inspections at www.buildingpermits.oregon.gov

Call or text the word "schedule" to 1-888-299-2821 use IVR number: 311095423292

Schedule using the Oregon ePermitting Inspection App, search “epermitting” in the app store

PERMIT FEES

Fee Description	Quantity	Fee Amount
Air conditioner	1	$11.90
Clothes dryer exhaust	1	$8.95
Decorative gas fireplace	1	$8.95
Ductwork – no appliance/fixture	3	$47.55
Flue vent for water heater or gas fireplace	2	$12.00
Furnace - up to 100,000 BTU	1	$11.90
Gas fuel piping outlets	6	$6.10
Heat pump	1	$8.95
Mechanical plan review		$46.51
Radon mitigation	1	$15.85
Range hood/other kitchen equipment	1	$8.95
Ventilation fan connected to single duct	6	$36.00
Water heater	1	$8.95
Single Family Residence - Baths	2	$316.75
Erosion Control Fee - New	1	$450.00
Excavation Fee in Right of Way	1	$51.50
Light & Power Service Extension	1	$450.00
Residential site review - new	1	$411.00
Street Tree Deposit	1	$439.00
Parks system development fee	1	$6,010.00
Residential sewer system development fee, part 1	1	$1,325.00
Residential sewer system development fee, part 2	1	$5,300.00
Surface water quantity	1	$352.55
Transportation development tax - residential	9998	$9,998.00
Water service SDC (3/4" meter)	1	$7,963.00
Water service-drop in connection fee (3/4" meter)	1	$368.00
Structural building permit fee		$2,058.15
Structural plan review fee		$1,337.80
State of Oregon Surcharge - Bldg (12% of applicable fees)		$246.98
State of Oregon Surcharge -Plumb (12% of applicable fees)		$38.01
State of Oregon Surcharge - Mech (12% of applicable fees)		$22.33
CET - Forest Grove School District - Res Use		$1,783.68
CET - Forest Grove School District - Admin Fee - Res Use		$74.32
Metro Construction Excise Tax		$367.02
Metro Construction Excise Tax - Admin Fee		$19.32
	Total Fees:	$39,614.97

Note: This may not include all the fees required for this project.

VALUATION INFORMATION

Construction Type	Occupancy Type	Unit Amount	Unit	Unit Cost	Job Value
VB	R-3 1 & 2 family	1,858.00	Sq Ft	$150.87	$280,316.46
VB	U Utility, misc.	567.00	Sq Ft	$60.43	$34,263.81
VB	U Utility, misc. - half rate	244.00	Sq Ft	$30.22	$7,373.68
				Total Job Value:	$321,953.95

ADDITIONAL INFORMATION/CONDITIONS OF APPROVAL FOR PUBLIC WORKS

Date Applied: 04/04/2023

Comments: • ALL HAUL-OFF OF ONSITE EXCAVATED MATERIALS SHALL BE DELIVERED TO AN APPROVED DESTINATION. IF THE DESTINATION IS OUTSIDE OF THE CITY OF FOREST GROVE, THE APPLICANT SHALL SUBMIT A COPY OF THE APPROVED PERMIT FROM THE GOVERNING LAND USE AUTHORITY OF THAT DESTINATION SITE. • BUILDER TO VERIFY ALL UTILITIES AND COMPLY WITH CURRENT CLEAN WATER SERVICES REGULATIONS. • INSTALL CONCRETE SIDEWALK AND DRIVE APPROACH PER CITY STANDARD SPECIFICATIONS. ALL STANDARD SPECIFICATIONS CAN BE LOCATED ONLINE AT:
HTTPS://WWW.FORESTGROVE-OR.GOV/ENGINEERING/PAGE/CONSTRUCTION-DESIGN-STANDARD-DETAILS • MAXIMUM DRIVEWAY WIDTH AT THE STREET RIGHT-OF-WAY SHALL BE 24' IN ALL RESIDENTIAL ZONES. IF FRONTAGE IS AT LEAST 60', DRIVEWAY WIDTH UP TO 30' MAY BE ALLOWED. • INSTALL EROSION CONTROL MEASURES AS REQUIRED PER CLEAN WATER SERVICES STANDARD SPECIFICATIONS.
HTTP://WWW.CLEANWATERSERVICES.ORG/PERMITS-DEVELOPMENT/EROSION-CONTROL/

ADDITIONAL INFORMATION/CONDITIONS OF APPROVAL FOR PLANNING

Date Applied: 04/03/2023

Comments: CONSTRUCTION ACCESS: INSTALL CONSTRUCTION ACCESS - 8" MINIMUM DEPTH BASE ROCK PAD, 20' MINIMUM LENGTH AND WIDTH. DO NOT TRACK MUD ONTO STREETS. THE STREET ADDRESS SHALL BE DISPLAYED IN A PROMINENT POSITION NEAR THE ENTRANCE TO THE BUILDING. NUMBERS SHALL BE AT LEAST 4 INCHES HIGH, OF A CONTRASTING COLOR AND VISIBLE FROM THE STREET. MUNICIPAL CODE SECTION 150.098 ONE STREET TREE TO BE PLANTED IN PARKWAY. INSTALLATION TO BE COMPLETED BY THE CITY. (THE STREET TREE DEPOSIT WILL BE REFUNDED FOR ANY TREE NOT INSTALLED. IF ADDITIONAL TREES ARE INSTALLED, THE APPLICANT WILL BE BILLED FOR THE DIFFERENCE.)

ADDITIONAL INFORMATION/CONDITIONS OF APPROVAL FOR UTILITY

Date Applied: 04/12/2023

Comments: General Contractor: See Electrical Site Plan from the Light & Power Department; for important information, requirements & comments regarding electric service installation. Pass all needed information on to electrical, excavation & other contractors.

For Sidewalk & Approach inspections, call the IVR system at 1-888-299-2821. Questions? Contact City Engineer's Office at 503-992-3228

Wet weather starts October 1st - May 31st. All applicable erosion control measures must be used

Any changes to approved plans require 2 copies of plan sized revisions to be submitted to and approved by the city. All revisions must be paid for and on site prior to inspections. Minimum charge is 1 hour time.

Erosion control is required at all times;
Please call 1-888-299-2821 to schedule at foundation inspection.

Eaves cannot extend greater than 12" into a 3 ft. setback
Per Table R302.1

All wood with less than 6 inches clearance from the ground to be rated for ground contact per R317.1(1-7)

Gutters, downspouts, rain drain piping is required. Storm water to disperse to a city approved storm drainage system

50% of all permanently installed interior and exterior lighting fixtures on the building contain high efficiency lamps
(min. 40 lumens per watt)

Electrical permits/inspections through Washington County Building Department.
503-846-3470

Carbon monoxide alarms are required per R315

Radon control methods to be incorporated per Section AF103.5.2 and Section AF103.6

All work & materials per 2021 edition of the Oregon Residential Specialty Code

Final grading - surface water shall not dispense onto adjacent properties

FRONT ELEVATION

1/4"=1'-0"

REAR ELEVATION

1/4"=1'-0"

311-23-000206-DWL

RECEIVED
03/31/2023
City of Forest Grove

3730 Tiana Street

Forest Grove Building Division

APPROVED

These plans have been reviewed for code compliance with all applicable codes and are approved for construction

Reviewer EVAN DAHL

Date 04/20/2023

Errors and omissions by the reviewer or inspector do not relieve the applicant from compliance with the codes

STONE BRIDGE HOMES NW

JOB:	FARMSTEAD 15
DRAWN BY:	PM
DATE:	3/9/23
SCALE:	1/4"=1'-0"
FILE:	3150-FC15

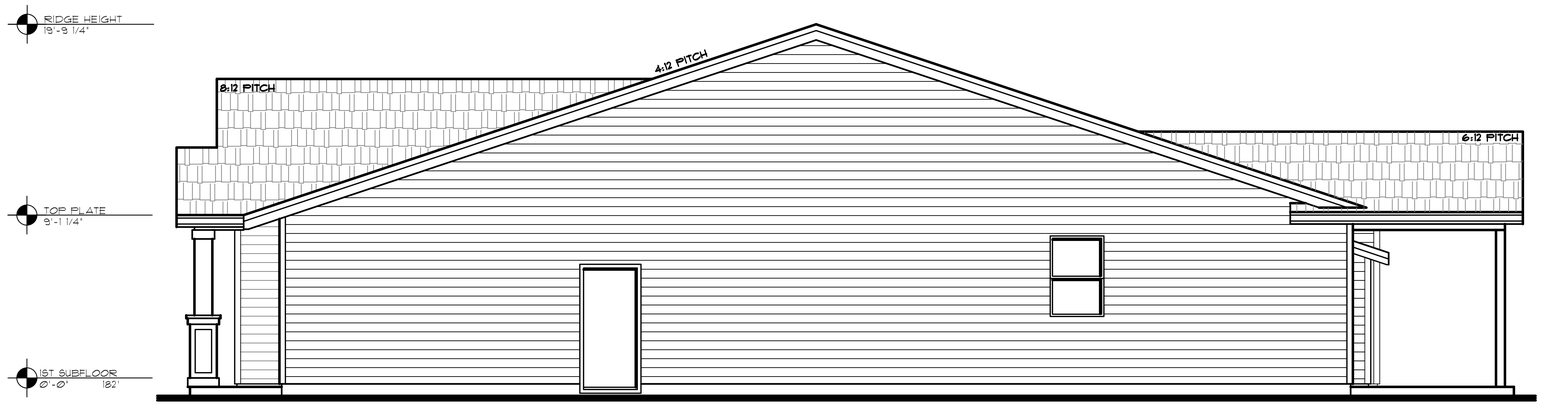
DRAWINGS:

EXTERIOR ELEVATIONS

SHEET No.

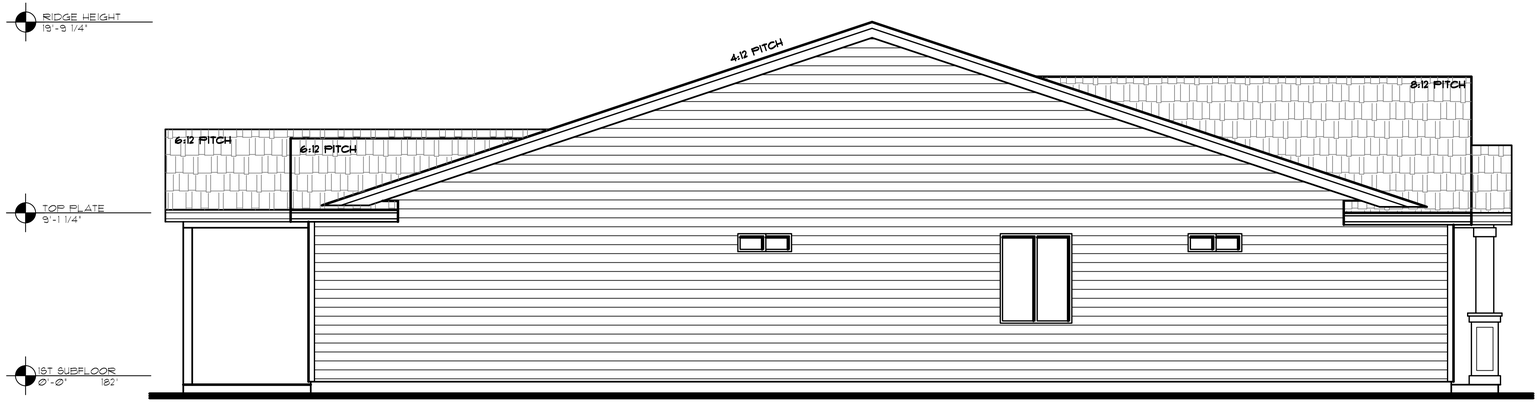
001

300
STANDARD
1,858 SQ.FT.



RIGHT ELEVATION

1/4"=1'-0"



LEFT ELEVATION

1/4"=1'-0"

300
STANDARD
1,858 SQ.FT.

STONE BRIDGE HOMES NW

JOB:	FARMSTEAD 15
DRAWN BY:	FM
DATE:	3/9/23
SCALE:	1/4"=1'-0"
FILE:	3150-FC15

DRAWINGS:

EXTERIOR ELEVATIONS

SHEET No.

002

*SQUARE FOOTAGE IS AN ESTIMATED FIGURE, IT MAY VARY.

*ALL DIMENSIONS ARE ESTIMATES, SOME MAY VARY.

*PLANS ARE DESIGNED FOR FLAT LOTS. IF MASONRY IS INCLUDED IN PLAN, THERE IS AN ADDITIONAL CHARGE IF HOME SITE IS NOT FLAT.

*ARCHITECURAL DRAWINGS ARE ESTIMATES OF HOW HOME WILL LOOK.

GENERAL NOTES:

- 9'-Ø" CEILING THRU-OUT MAIN FLOOR PLAN

- 4 X 10 HEADERS TYP. UNLESS NOTED OTHERWISE
- VOIDS IN HEADERS TO BE INSULATED W/ MIN. R-4 RIGID INSULATION.
- NON-STRUCTURAL HEADERS MAY BE REPLACED ENTIRELY W/RIGID INSULATION

- FULL-HEIGHT STUDS REQUIRED AT EACH END OF HEADERS PER R602.7.3
  - LESS THAN 3'-Ø" = 1 STUD
  - LESS THAN 4'-Ø" = 2 STUDS
  - LESS THAN 8'-Ø" = 3 STUDS
  - LESS THAN 12'-Ø" = 5 STUDS
  - LESS THAN 16'-Ø" = 6 STUDS

-INTERMEDIATE FRAMING FOR EXTERIOR WALLS TYP.; 2 X 6 STUDS @ 16" O.C. W/ R-23 BLOWN INSULATION
- CORNERS 4 INTERSECTIONS, TYP., TO BE FULLY INSULATED THRU 2- OR 3- STUD CORNER TECHNIQUE PER N1104.3.2

- INTERMEDIATE FRAMING FOR INTERIOR WALLS, TYP.; 2 X 4 STUDS @ 16" O.C. INTERSECTIONS W/ EXTERIOR WALLS TO BE FULLY INSULATED PER N1104.3.2

- ALL NEW, PERMANENTLY INSTALLED INTERIOR & EXTERIOR LIGHTING FIXTURES CONTAIN HIGH EFFICIENCY LAMPS

- NEW WINDOWS SHALL HAVE U-VALUES OF Ø.27 OR LESS

- NEW EXTERIOR DOORS SHALL HAVE U-VALUES OF Ø.20 OR LESS

- APPROX. 7'-Ø" HEADER HEIGHT THRU-OUT MAIN FLOOR (83+")

- 8'Ø" TALL FRONT DOOR HEADER HEIGHT APPROX. 8'-3" (99+")

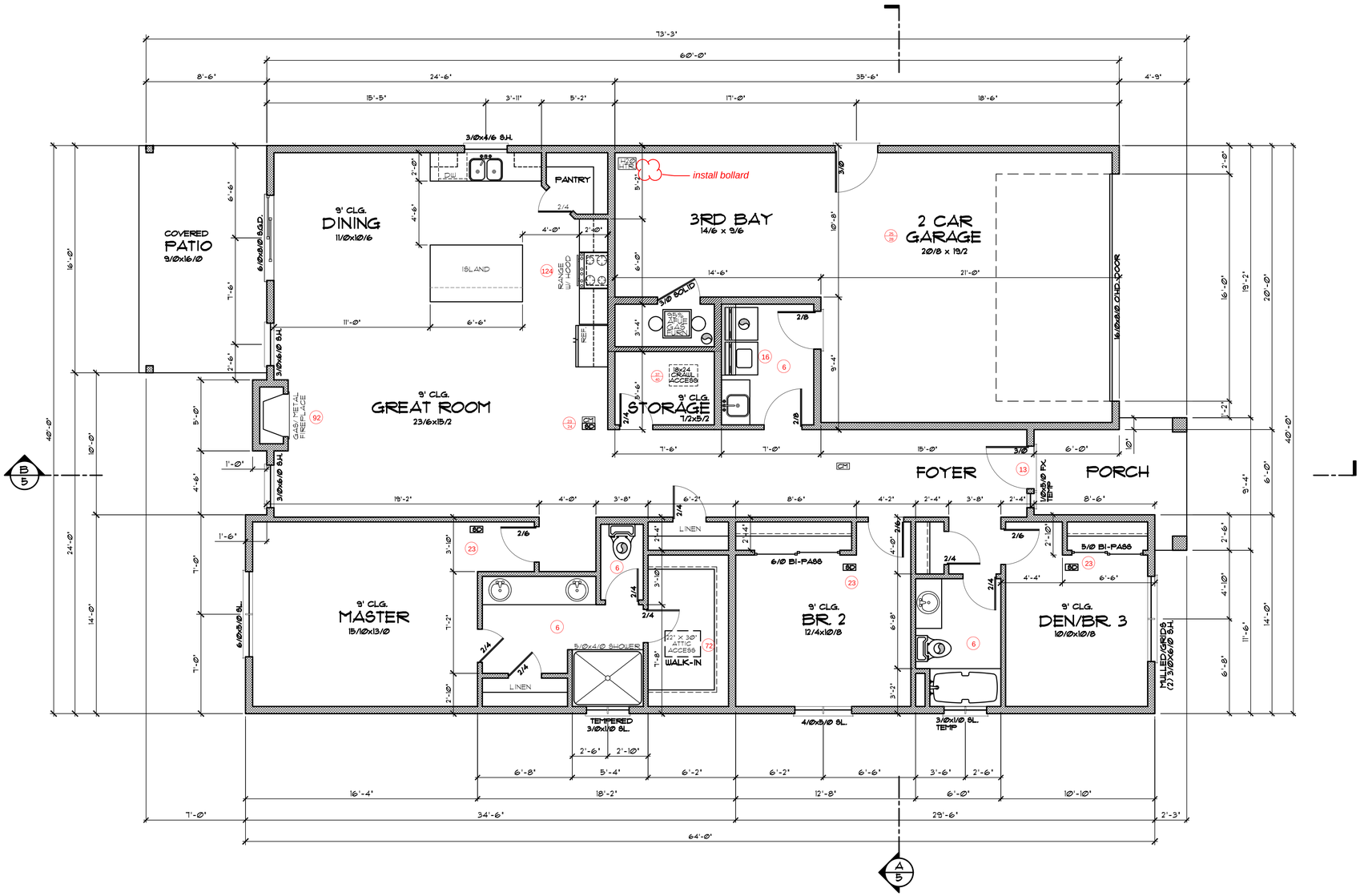
TABLE N1104.1.2
ADDITIONAL ENERGY EFFICIENCY MEASURE TO BE INSTALLED:

HIGH EFFICIENCY HVAC SYSTEM: GAS-FIRED FURNACE AFUE 94% OR BETTER

SD SMOKE DETECTOR
CO CARBON MONOXIDE DETECTOR

CFM 50 CFM FAN HARD-DUCTED TO EXTERIOR

TWH TANKLESS WATER HEATER DIRECT VENT TO EXTERIOR



MAIN FLOOR PLAN

1/4"=1'-Ø"

1,858 SQ. FT.
GARAGE 561 SQ. FT.

300
STANDARD
1,858 SQ.FT.

STONE BRIDGE HOMES NW

JOB:	FARMSTEAD 15
DRAWN BY:	FM
DATE:	3/9/23
SCALE:	1/4"=1'-Ø"
FILE:	315Ø-FC15

DRAWINGS:

FLOOR PLAN

SHEET No.

003

FOUNDATION NOTES:

A 15"X1' CONCRETE FOOTING
12"X1' CONCRETE FOOTING

B 8" CONCRETE FOUNDATION WALL
6" CONCRETE FOUNDATION WALL

C COVER ENTIRE CRAWL SPACE WITH 6 MIL. BLACK VISQUEEN VAPOR BARRIER

D PROVIDE 18"X6" SCREENED VENTS, COORDINATE LOCATIONS TO NOT FALL UNDER FLOOR BEAM LOCATIONS (AS SHOWN) OR UNDER ANY HEADER SUPPORTS FROM MAIN FLOOR HEADERS (TYP.) PER ORSC 408.1: A MINIMUM OF 1 NET SQFT. OF VENTILATION PER EACH 150 SQFT. OF UNDER-FLOOR AREA IS REQUIRED

E PROVIDE A CRAWL SPACE LOW POINT FND. DRAIN PER U.B.C. (INCLUDE BACKCHECK VALVE)

F PROVIDE A 1/2" AIR SPACE @ ENDS AND SIDES OF BEAMS @ CONC. FOUNDATION WALL, SET BEAMS ON 55# A.S. FELT AND PROVIDE 3" MIN. BEARING

G 4X8 BEAM @ 2'-8" O.C. ON 4X4 POSTS (4X6 POSTS @ BEAM SPLICES), ON 55# FELT ON 18"ϕ X 8" CONCRETE FOOTINGS

H 4" SLAB ON GRADE AT GARAGE SLOPE SLAB MIN. 4" TO GARAGE DOOR(S)

I CONCRETE SLAB AT PORCH

J (NOT USED) 2X6 STUD WALL ON 12X1' CONCRETE FOOTING

- INSULATION:

R-38 BATT INSUL. IN CRAWL SPACE
R-8 DUCT INSULATION & MASTIC-SEALED

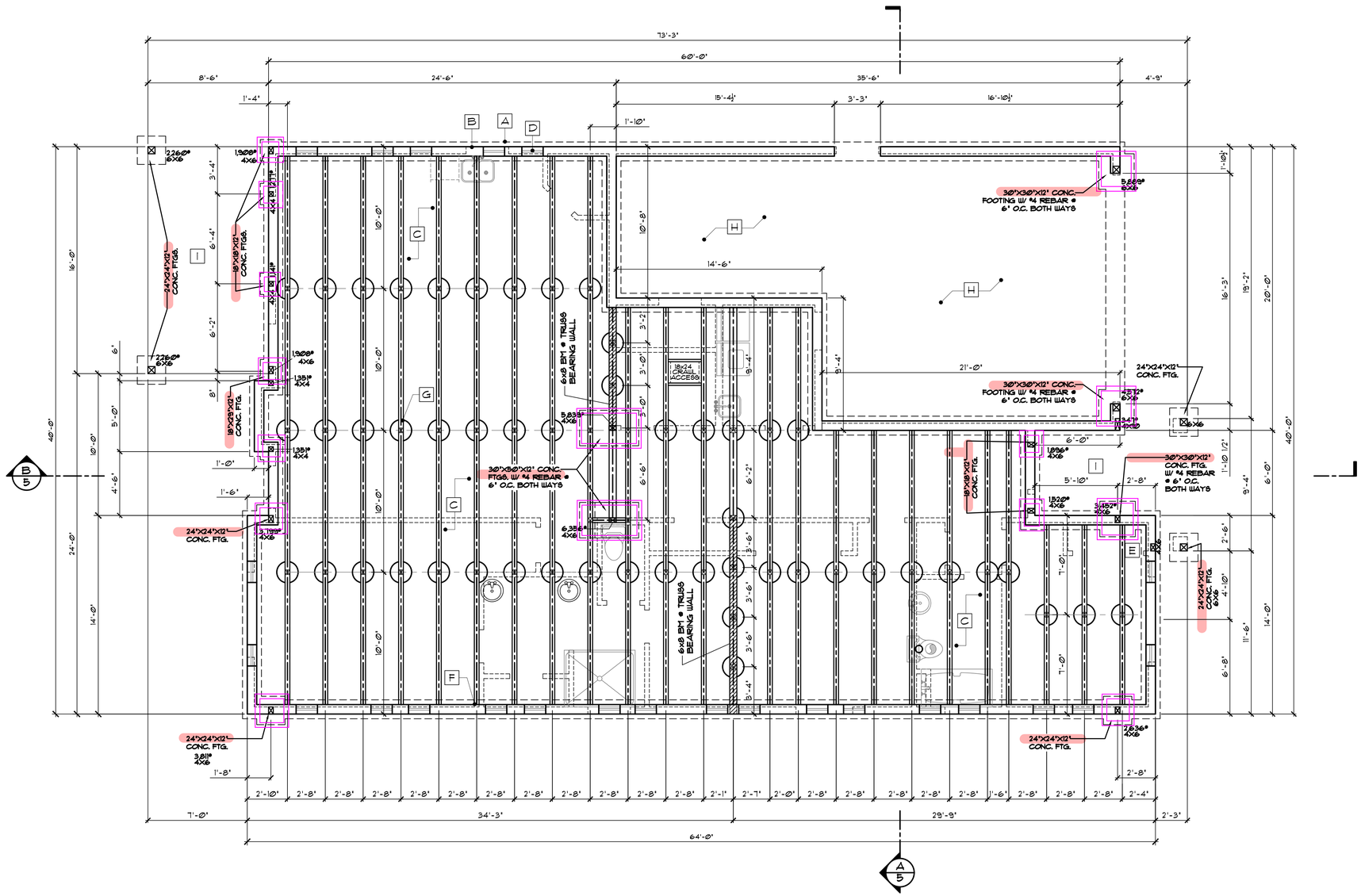
⊠ BEARING POINT TO BE 4X4 POST MIN. (TYP. UNO)

VENTILATION CALCS.	
CRAWL	1,720 SQFT. CRAWL SPACE AREA
REQ'D	1/150 = 11.46 SQFT. VENTING REQ'D
VENTS	11.46/0.47 = 24 FOUND. VENTS REQ'D MIN.

*17 FOUNDATION VENTS PROVIDED PER PLAN

RADON MITIGATION

SEE GENERAL NOTES FOR RADON MITIGATION REQUIREMENTS.



FOUNDATION PLAN

1/4"=1'-0"

Foundation wall vents required within 3' of foundation wall corners

PROVIDE LOW-POINT CRAWL SPACE DRAIN

MIN. (2) #4 HORZ. REBAR (cont.) IN FOOTING & (1) #4 HORZ REBAR (cont.) IN UPPER 12" OF FOUNDATION WALLS (U.N.O.)

CONCRETE WORK
29-36

GARAGES
25-28

CRAWL SPACE
37-40

STONE BRIDGE HOMES NW

JOB:	FARMSTEAD 15
DRAWN BY:	FM
DATE:	3/9/23
SCALE:	1/4"=1'-0"
FILE:	3150-FC15

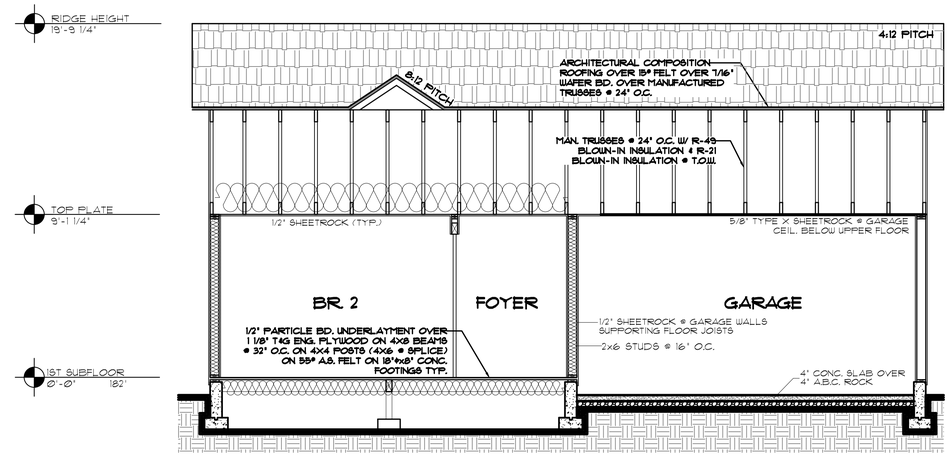
DRAWINGS:

FOUNDATION PLAN

SHEET No.

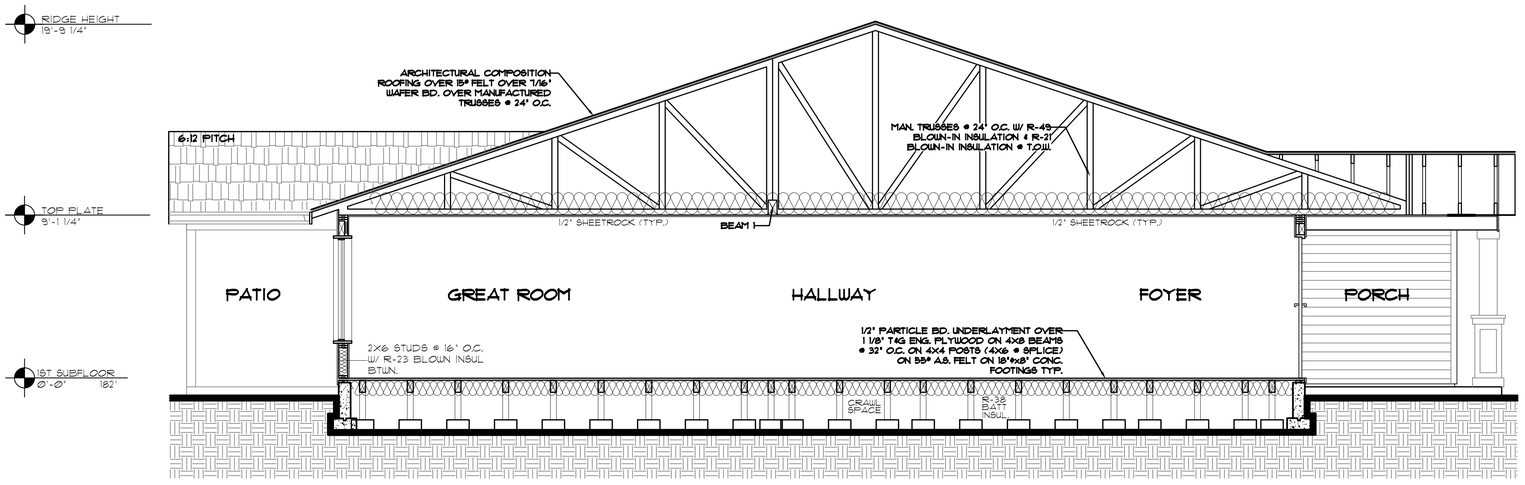
004

300
STANDARD
1,858 SQ.FT.



SECTION A

1/4"=1'-0"



SECTION B

1/4"=1'-0"

300
STANDARD
1,858 SQ.FT.

STONE BRIDGE HOMES NW

JOB:	FARMSTEAD 15
DRAWN BY:	FM
DATE:	3/9/23
SCALE:	1/4"=1'-0"
FILE:	3150-FC15

DRAWINGS:

BUILDING SECTIONS

SHEET No.

005

ROOF FRAMING NOTES:

-ROOF TO BE FRAMED WITH RAISED HEEL PRE-MANUF'D ROOF TRUSSES @ 24" O.C. EXCEPT WHERE NOTED OTHERWISE.

- ALL SAWN LUMBER FRAMING MEMBERS (BEAMS, HEADERS, RAFTERS) TO BE DOUGLAS FIR LARCH NO. 2 OR BETTER, TYP. UNLESS SPECIFIED OTHERWISE.

- INSULATION:
2-4@ BLOWN INSUL. IN FLAT AREAS
R-30 BLOWN INSUL. IN VAULTED AREAS PROVIDE FULL INSULATING VALUE AT OUTSIDE OF EXTERIOR WALLS W/ RIGID INSULATION
R-21 BLOWN INSUL. @ TOP OF WALLS
R-8 DUCT INSUL. & MASTIC-SEALED

- ROOF TO BE PANELED W/ 1/2" O.S.B. ROOF SHEATHING. MAINTAIN AN 1/8" GAP BETWEEN PANELS.

- EAVES TO BE PANELED W/ 1/2" PLYWOOD, AT ALL EXPOSED EAVES.

- GALVANIZED GUTTER SYSTEM

-USED HURRICANE TIES AT TRUSS TO TOP PLATE CONNECTIONS

-30 YEAR ARCHITECTURAL COMP. ROOFING (TYP.)

' 4 X 10 HEADERS TYP. UNLESS NOTED OTHERWISE
VOIDS IN HEADERS TO BE INSULATED W/ MIN. R-4 RIGID INSULATION NON-STRUCTURAL HEADERS MAY BE REPLACED ENTIRELY W/ RIGID INSULATION.

- ALL BATH AND UTILITY ROOM EXHAUST FANS MUST BE HARD-DUCTED TO EXTERIOR THRU ADD'L ROOF VENTS AS SHOWN ON PLAN

- VENTILATE ALL AREAS OF ROOF BETWEEN MAIN ROOF BODY AND OVER-FRAMING

- ATTIC VENTIALTION AS FOLLOWS PER ORSC R806.2:
-A MIN. OF 1 NET SQ.FT. OF VENTILATION PER 300 SQ.FT. OF ATTIC AREA IS REQUIRED
-REQUIRED VENTILATION OPENINGS SHALL OPEN DIRECTLY TO THE OUTSIDE AIR
-INSTALL CLASS II VAPOR BARRIER, PRIME PERM, TO "WARM-IN-WINTER" SIDE OF CEILING

- EAVE VENTS TO BE INSTALLED IN EVERY TRUSS BAY AT COVERED DECK
- ROOF VENTS PROVIDE @ 347 SQ.FT. VENTILATION
- EAVE VENTS PROVIDE @ 291 SQ.FT. VENTILATION

INDICATES ROOF FRAMED OVER ROOF BELOW: USE 2X6 RAFTERS @ 24" O.C. W/ 2X10 RIDGES, & 2X10 VALLEY RAFTERS LAID FLAT ON TRUSSES BELOW

INTERIOR BEARING WALL DETAIL

7"X7" VENTS AT RIDGE AS SHOWN

EXHAUST FAN ROOF VENTS AS SHOWN

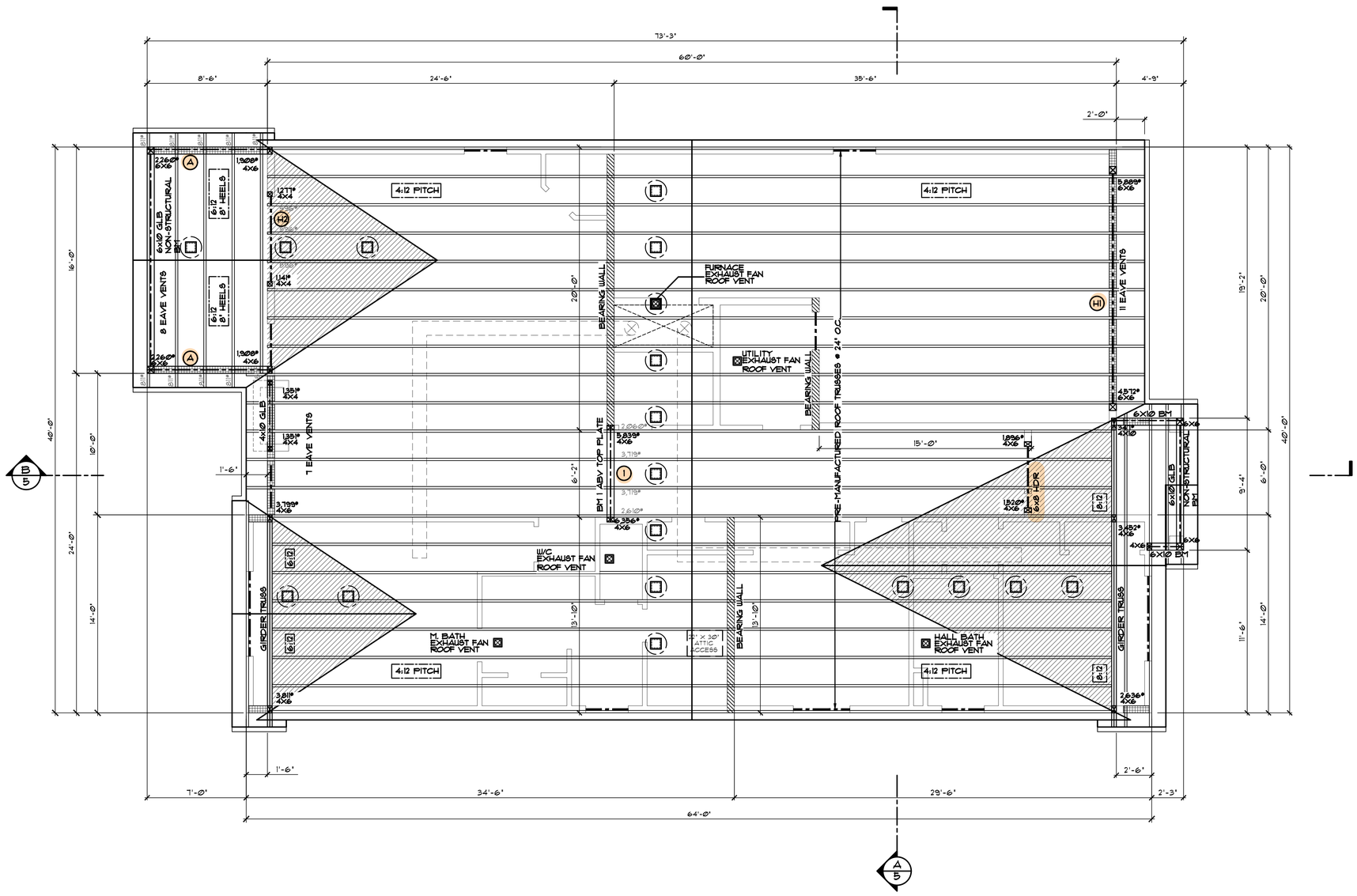
EAVE VENTS AS SHOWN

VENTILATION CALCS.	
ATTIC	2,461 SQ.FT. UPPER FLR. ATTIC AREA
VENTING	1/300= 8.02 SQ.FT. VENTING REQ'D
70/30 SPLIT	5.6/@.347= 16 ROOF VENTS REQ'D MIN.
	2.4/@.291= 24 EAVE VENTS REQ'D MIN.

*17 ROOF VENTS + 5 EXHAUST VENTS ON ROOF (22 TOTAL)
26 EAVE VENTS ON ROOF

BEAM SCHEDULE	
H1	5 1/2" x 14" GLB HDR.
H2	4x10 D FIR-L #2 HDR.
A	5 1/2" x 9 1/2" GLB (QTY. 2)
1	5 1/2" x 9 1/2" GLB

(H1) BEAM SCHEDULE FLAG



ROOF FRAMING PLAN

1/4"=1'-0"

99

300
STANDARD
1,858 SQ.FT.

STONE BRIDGE HOMES NW

JOB:	FARMSTEAD 15
DRAWN BY:	PM
DATE:	3/9/23
SCALE:	1/4"=1'-0"
FILE:	3150-FC15

DRAWINGS:

ROOF FRAMING PLAN

SHEET No.

006

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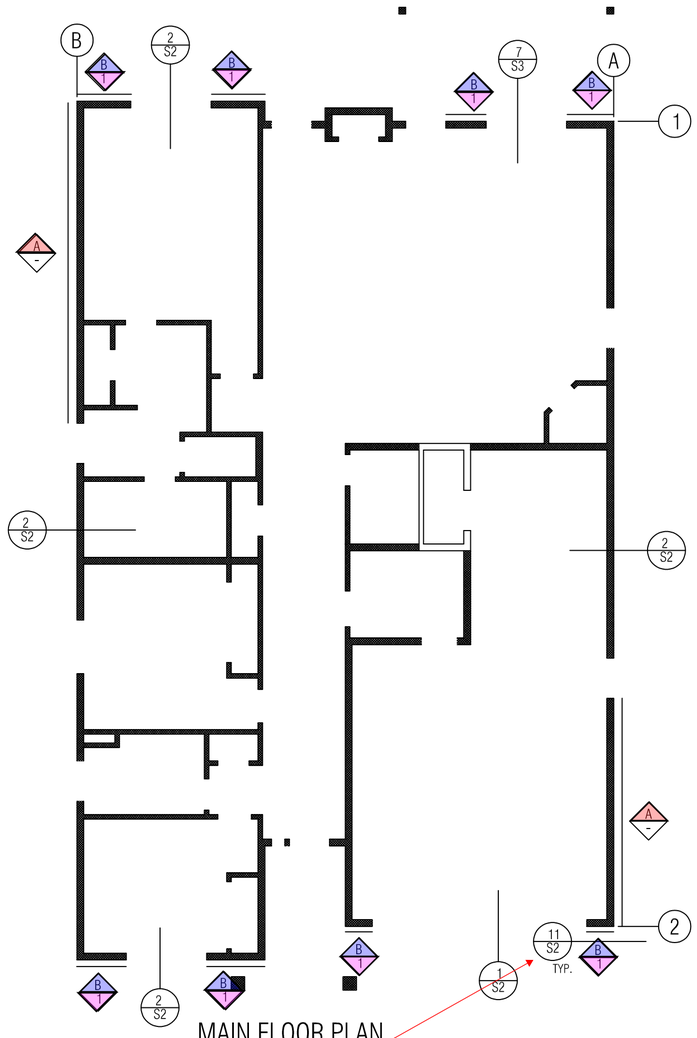
(208)850-7542 mike.brown@core-engineering.net

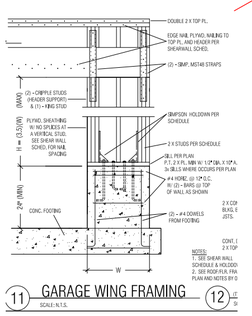
STRUCTURAL REGISTERED PROFESSIONAL ENGINEER
71321PE
OREGON
SEPT. 10, 2004
MICHAEL KINGSLEY BROWN

EXPIRATION DATE: 12-31-23



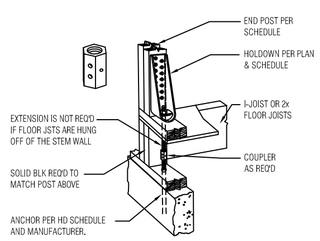
MAIN FLOOR PLAN

SCALE: 1/8"=1'



11 GARAGE WING FRAMING

SCALE: N.T.S.



2 TYPICAL HOLDOWN DETAIL

SCALE: N.T.S.

LEGEND

EXTENT OF SHEARWALL SHEATHING SEGMENT

INDICATES SHEARWALL TYPE

A -

INDICATES HOLDOWN TYPE. (1) ON EA END OF WALL SEGMENT

HOLDOWN SCHEDULE

MARK NUMBER	HOLDOWN	Boundary Studs	Tension of DF Allowable Lbs	Tension of HF Allowable Lbs	Anchor Mono Pour	Anchor Two Pour
1	HDU2-SDS2.5 OR PHD2-SDS3	(2)2x	3075	2215	SSTB24	SSTB24
2	HDU4-SDS2.5 OR PHD5-SDS3	(2)2x	4545	3270	SB5/8x24	SB5/8x24
3	HDU8-SDS2.5	(3)2x	7870	5665	PAB7	7/8"DIA.x24" MIN EMBED INTO CONC. A36 THREADED ROD w/ HEAVY SQ. NUT AND BP7/8-2 BEARING PLATE WASHER @ BOTT.
4	HDU11-SDS2.5	(1)6x	9535	6865	1"DIA.x42" A36 THREADED ROD w/ HEAVY SQ. NUT AND WASHER @ BOT. PROVIDE 24" EMBED INTO CONT FTG.	1"DIA.x42" A36 THREADED ROD w/ HEAVY SQ. NUT AND WASHER @ BOT. PROVIDE 24" EMBED INTO CONT FTG.
8	HHDQ14-SDS2.5	(1)6x	13710	10745		
5	MSTC28	(2)2x	3000	2590	N/A	N/A
6	MSTC40	(2)2x	4335	3745	N/A	N/A
7	MSTC66	(2)2x	5660	5660	N/A	N/A

Notes:
1. Install all holdowns per manufactureer specificaiton per latest Simpson Strong Tie catalog.
2. Match studs on schedule for walls below on all wall to wall holdowns.
3. (Multiple)2x studs nailed together with (2) rows of 16d @ 3" o.c. staggered.
4. Refer to shearwall schedule and typical shearwall details for wall locations and configurations.
5. Refer to Simpson catalog for minimum embed of anchors into concrete.

SHEARWALL SCHEDULE (a-m)

MARK NUMBER	REF NOTES: (a,i) SHEATHING	Note: (b) NAIL SIZE	EDGE NAILING SPACING	FEILD NAILING SPACING	SILL TO CONCRETE CONNECTION. Note: (c)	SILL TO WOOD CONNECTION. Note (g)	SHEAR TRNASFER CLIPS (h)	Lbs CAPACITY
A	7/16" OSB (1) SIDE	8d	6"	12"	1/2" Dia. A.B. @ 48" o/c	16d @ 6" o/c	A35 @ 24" o/c	280
B	7/16" OSB (1) SIDE (f)	8d	4"	12"	1/2" Dia. A.B. @ 24" o/c	16d @ 4" o/c	A35 @ 14" o/c	520
C	7/16" OSB (1) SIDE (e,f)	8d	3"	12"	1/2" Dia. A.B. @ 18" o/c (m)	16d @ 3" o/c	A35 @ 11" o/c	720
D	7/16" OSB (1) SIDE (e,f)	8d	2"	12"	1/2" Dia. A.B. @ 16" o/c (m)	16d @ 2" o/c	A35 @ 8" o/c	936
E	7/16" OSB (2) SIDE (d,e,f)	8d	6"	12"	1/2" Dia. A.B. @ 18" o/c (m)	16d @ 3" o/c	A35 @ 10" o/c	768
F	7/16" OSB (2) SIDE (d,e,f)	8d	4" Staggered	12"	1/2" Dia. A.B. @ 12" o/c (m)	16d @ 2" o/c	A35 @ 7" o/c	1120
G	7/16" OSB (2) SIDE (d,e,f)	8d	3" Staggered	12"	1/2" Dia. A.B. @ 10" o/c (m)	16d @ 3" o/c(2)rows staggered	A35 @ 5" o/c	1440
H	7/16" OSB (2) SIDE (d,e,f)	8d	2" Staggered	12"	1/2" Dia. A.B. @ 8" o/c (m)	16d @ 2" o/c(2)rows staggered	HGA10KT @ 7" o/c	1872

Notes:
a) Wood structural panels shall conform to the requireents for its type in DOC PS 1 or PS 2. All wall construction to conform to AF&PA SDPWS per IBC 2306.3.
b) Use Common Wire Nails for all wood sheathing and cooler nails for gypboard sheathing.
c) A.B. minimum 7" embed into concrete. 3"x3"x1/4" plate washers req'd at all shear wall A.B. Locate edge of Washer no greater than 1/2" from sheathed edge of sill plate.
d) Panel joints shall be offset to fall on different framing members or framing shall be 3x or thicker and nails on each side shall be staggered.
e) 3x or Dbl 2x framing at all panel edges and nails shall be staggered.
f) All edges blocked.
g) Common Wire Nails.
h) Clip to be attached from continuous blocking to top of continuous top plates.
Clips are not required at Gyp Bd walls but blocking is attached per the toenailing schedule.
i) See attached typical shearwall details.
j) Sheathing to be Structrual I Sheathing.
k) Values are for framing of H-F.
m) See plan for walls where seismic design shear is greater than 560 plf. 3x or 2x flat blkg at panel edges, Stagger nails.
Where indicated on the plans 3x sills are required with A.B. spacing as per plan and schedule. See note C for plate washers req'd at A.B.
For walls with the larger sills, Anchor Bolt spacing may be increased by a factor of 1.25 from the table above due to thicker sills.

DRAWN BY:	MKB
DATE:	03/28/23
SCALE:	N.T.S.
DMH JOB#:	3150
ENG JOB #	DM23-038
DRAWINGS:	LATERAL PLAN
SHEET No.	S1

MICHAEL K. BROWN, S.E.
REGISTERED PROFESSIONAL STRUCTURAL ENGINEER
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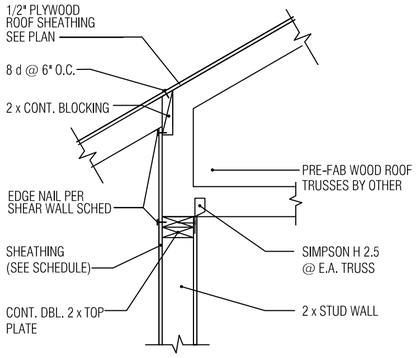
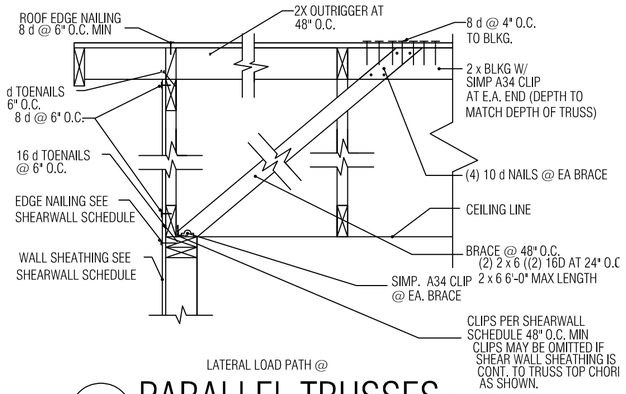
STRUCTURAL REGISTERED PROFESSIONAL ENGINEER
71321PE
OREGON
SEPT. 10, 2005
MICHAEL KINGSLEY BROWN

EXPIRATION DATE: 12-31-23



1 ROOF BLKG. @ TRUSSES

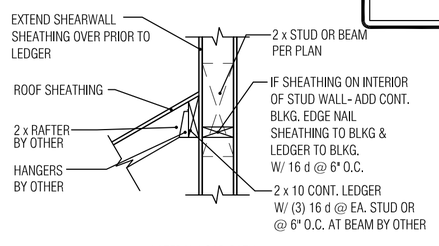
SCALE: N.T.S. SHEAR WALL



LATERAL LOAD PATH @

2 PARALLEL TRUSSES TO WALL

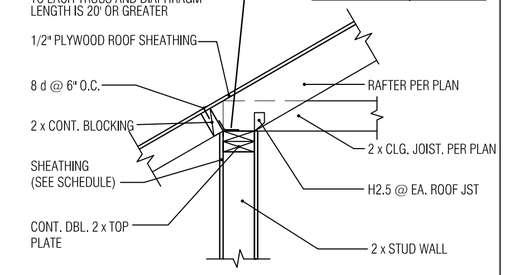
SCALE: N.T.S.



LATERAL LOAD PATH @

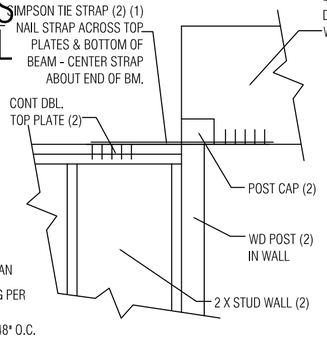
3 LEDGER DETAIL

SCALE: N.T.S. SHEAR WALL

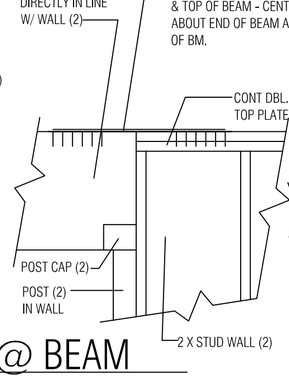


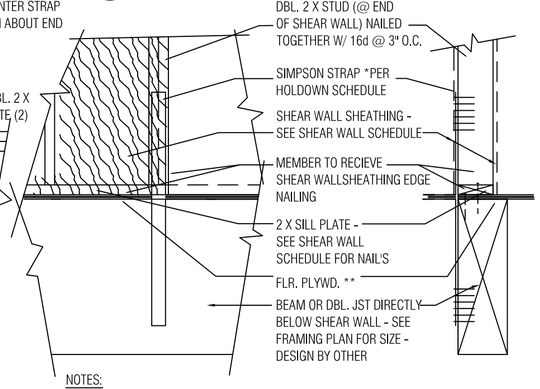
4 ROOF BLKG @ JOIST

SCALE: N.T.S. SHEAR WALL



NOTES:
1. SEE PLAN FOR STRAP SIZE SHOWN FOR CLARITY.
2. SEE ROOF/FLOOR FRAMING PLANS BY OTHER

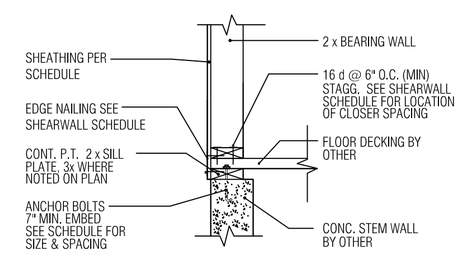




NOTES:
1. SEE SHEAR WALL SCHEDULE & HOLDDOWN SCHEDULE
2. SEE ROOF/FLR. FRAMING PLAN AND NOTES BY OTHER

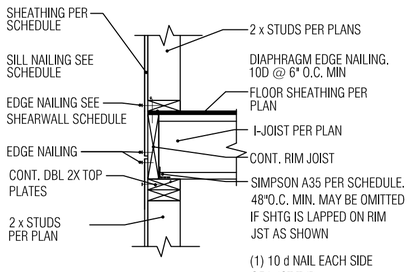
* CENTER STRAP ABOUT SILL PLATE

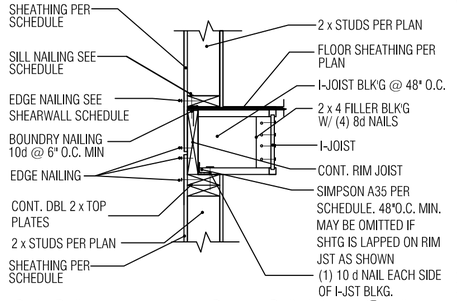
** 10d @ 6" O.C. EDGES 12" O.C. FIELD



5 PERP. FLR. FRAMING ALT.

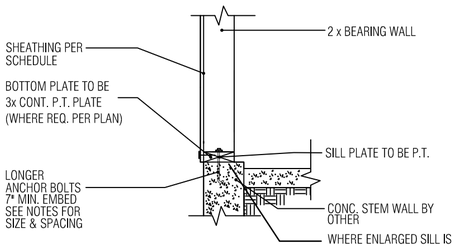
SCALE: N.T.S. @ EXT. WALL





6 PARALLEL FLR. FRAMING ALT.

SCALE: N.T.S. @ EXT. WALL

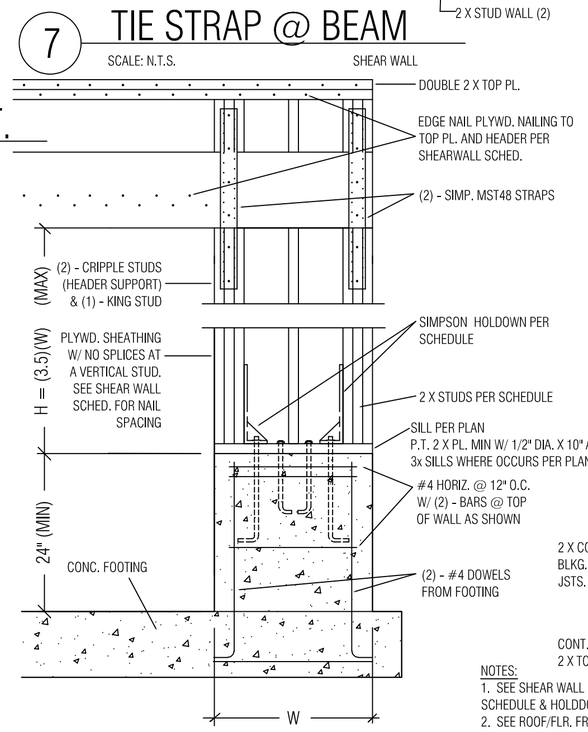


9 POST & BEAM STEM WALL

SCALE: N.T.S. SHEAR WALL

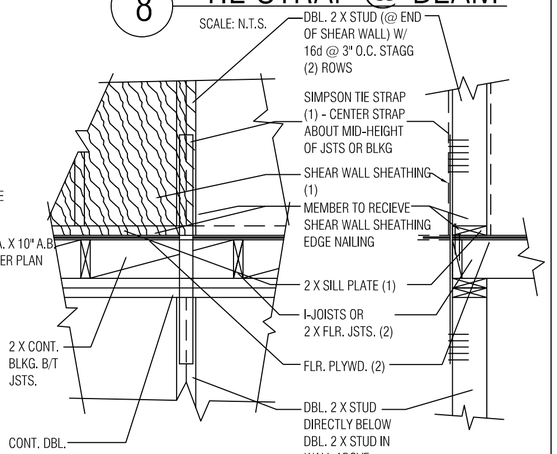
10 3 X SILLS AT SEIS WALLS

SCALE: N.T.S.



7 TIE STRAP @ BEAM

SCALE: N.T.S. SHEAR WALL



NOTES:
1. SEE SHEAR WALL SCHEDULE & HOLDDOWN SCHEDULE
2. SEE ROOF/FLR. FRAMING PLAN AND NOTES BY OTHER

8 TIE STRAP @ BEAM

SCALE: N.T.S.

11 GARAGE WING FRAMING

SCALE: N.T.S.

12 (TYP.) TIE STRAP HOLDDOWN

SCALE: N.T.S.

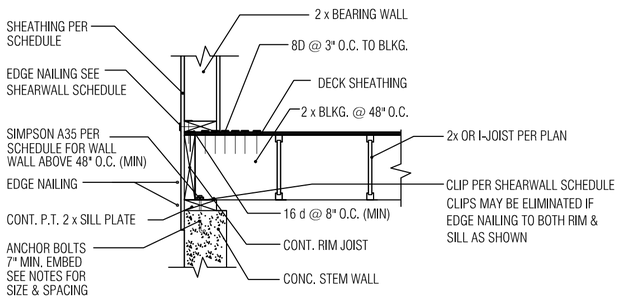
DRAWN BY:	MKB
DATE:	03/28/23
SCALE:	N.T.S.
DMH JOB#:	3150
ENG JOB #	DM23-038

DRAWINGS:

DETAILS

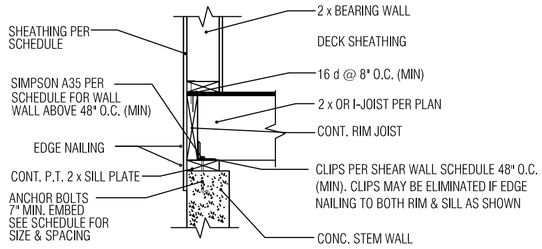
SHEET No.

S2



1 PARALLEL JOISTS

SCALE: N.T.S. SHEAR WALL



2 PERP JOISTS

SCALE: N.T.S. SHEAR WALL

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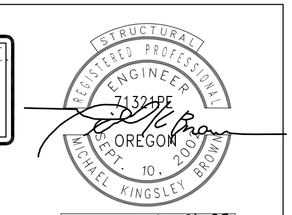
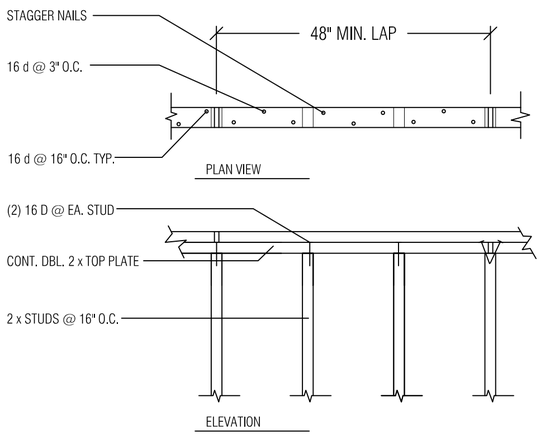
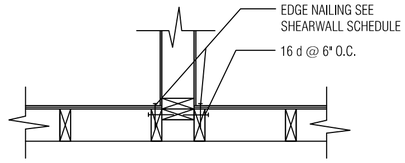


EXPIRATION DATE: 12-31-23



3 TOP PLATE SPLICE

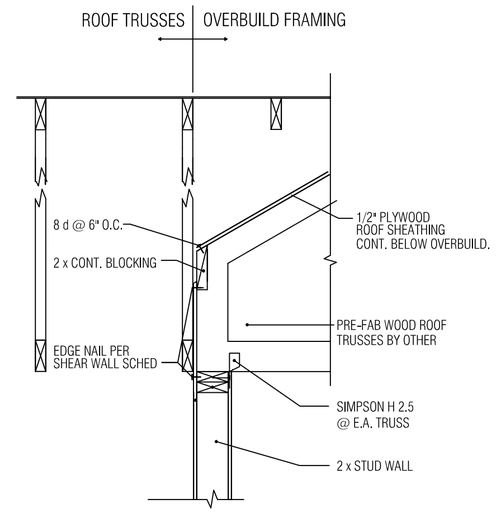
SCALE: N.T.S. SHEAR WALL

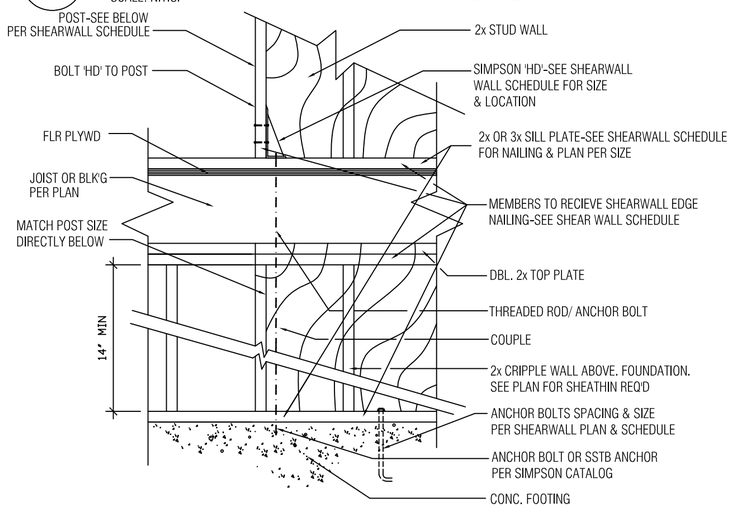


SHEAR TRANSFER

4 THRU WALL INTERRUPTION

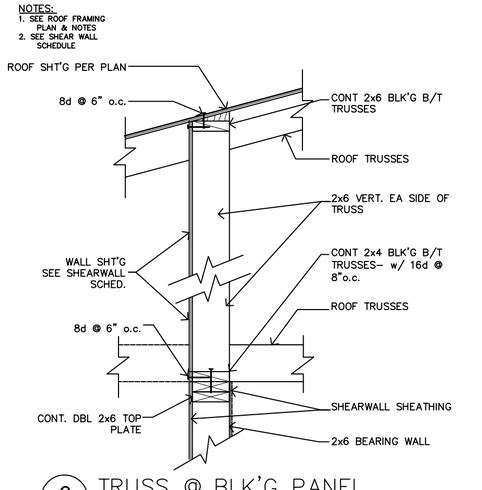
SCALE: N.T.S. SHEAR WALL





5 TYP. HOLDOWN & SHEARWALL @ CRIPPLE WALLS

SCALE: N.T.S.

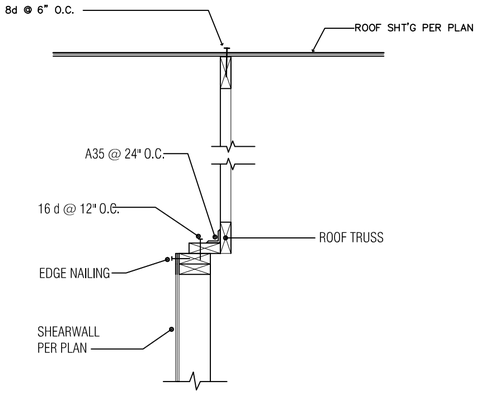


6 TRUSS @ BLK'G PANEL

SHEAR WALL SCALE: N.T.S.

7

SCALE: N.T.S.



8

SCALE: N.T.S.

DRAWN BY:	MKB
DATE:	03/28/23
SCALE:	N.T.S.
DMH JOB#:	3150
ENG JOB #	DM23-038

DRAWINGS:

DETAILS

SHEET No.

S3

GENERAL REQUIREMENTS

GENERAL REQUIREMENTS

1. THESE PLANS WERE DRAWN TO CONFORM TO THE 2021 OREGON RESIDENTIAL SPECIALTY CODE. FREQUENT CHANGES TO BUILDING CODES (EITHER LOCAL, STATE OR NATIONAL) AND/OR TO MECHANICAL/ ELECTRICAL/ PLUMBING AND ENVIRONMENTAL CODES MAY MAKE CONFORMITY TO ALL BUILDING CODES AND RESTRICTIONS IMPOSSIBLE. CONSEQUENTLY, THE USE OF THESE PLANS IS SUBJECT TO LOCAL CODE REQUIREMENTS AND INTERPRETATIONS.
2. PERSONS INVOLVED IN WORK ON PROJECT SHALL BE RESPONSIBLE FOR EXAMINATION OF BUILDING SITE, PLANS AND SPECIFICATIONS. FAILURE TO DO SO WILL NOT RELEASE BIDDERS FROM PERFORMING THE WORK INDICATED BY THE PLANS AND SPECIFICATIONS.
3. NO VARIATION REQUIRED BY A BUILDING OFFICIAL SHALL BE BINDING ON THE DESIGNER.
4. ALL IDEAS, ARRANGEMENTS, DRAWINGS AND PLANS SET FORTH ON THESE SHEETS ARE THE ORIGINAL WORK, THE PRODUCT, AND THE PROPERTY OF STONE BRIDGE HOMES NW, LLC. ALL USE OF THIS INFORMATION IS LIMITED TO A SPECIFIC PROJECT OF THE PURCHASER, AND FOR THE CONSTRUCTION OF ONE BUILDING. ANY USE, REUSE OR DISCLOSURE OF THESE PLANS, REPRODUCTIONS, IDEAS, DESIGNS, AND/OR ARRANGEMENTS OTHER THAN BY STONE BRIDGE HOMES NW, LLC, IS STRICTLY PROHIBITED BY LAW WITHOUT THE WRITTEN PERMISSION OF STONE BRIDGE HOMES NW, LLC.

DESIGN LOADS

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH THE LATEST EDITION OF THE OREGON RESIDENTIAL SPECIALTY CODE FOR ALL SPECIFICATIONS, FRAMING EXECUTIONS AND FOR VERIFICATION OF ALL DESIGN LOADS.

1. FLOORS: 40 P.S.F. LIVE LOAD
   10 P.S.F DEAD LOAD (AVERAGE)
2. ROOF: 25 P.S.F. LIVE LOAD
   A. COMPOSITION ROOFING: 10 P.S.F. DEAD LOAD (AVERAGE)
   B. SHAKE ROOFING: 17 P.S.F. DEAD LOAD (AVERAGE)
   C. CONCRETE TILE ROOFING: 20 P.S.F. DEAD LOAD (AVERAGE)
3. ATTICS:
   A. WITHOUT STORAGE: 10 P.S.F. LIVE LOAD
      5 P.S.F. DEAD LOAD (AVERAGE)
   B. LIMITED STORAGE: 20 P.S.F. LIVE LOAD
      10 P.S.F. DEAD LOAD (AVERAGE)
4. DECKS: 40 P.S.F. LIVE LOAD
   7 P.S.F. DEAD LOAD
5. STAIRS: 40 P.S.F. DEAD LOAD
   300 LB. CONCENTRATED LOAD
6. GARAGE FLOOR: 50 P.S.F. DEAD LOAD
   2000 LB. CONCENTRATED LOAD
7. GUARDS & HANDRAILS: 200 LB. CONCENTRATED LOAD
8. VARIATION FROM DESIGN LOADS STATED ABOVE SHALL BE DETERMINED AND VARIED THROUGH LOCAL AUTHORITIES AND MODIFICATION OF DESIGN APPROVED BY THOSE AUTHORITIES.

EXCAVATION AND GRADING

1. EXCAVATE FOR FOOTINGS TO UNDISTURBED AND SOLID EARTH. MINIMUM DEPTH SHALL BE 18" FOR ONE-STOREY AND TWO-STOREY STRUCTURES, OR 24" FOR THREE-STOREY STRUCTURES, UNLESS OTHERWISE NOTED OR DICTATED BY LOCAL AUTHORITIES.
2. REMOVE ALL LOOSE MATERIALS FROM EXCAVATIONS PRIOR TO POURING OF CONCRETE.
3. BACKFILL FOR POSITIVE SLOPE AWAY FROM THE STRUCTURE WITH SLOPE NO GREATER THAN 6 IN 12.
4. DO NOT EXCAVATE GREATER THAN A 2:1 SLOPE BELOW FOOTINGS.
5. MAINTAIN MINIMUM 6" SPACE FROM GROUND TO WOOD SIDING.
6. ACTUAL FINISHED GRADES MAY VARY SLIGHTLY FROM FINISHED GRADES SHOWN ON PLANS AND ELEVATIONS.

FOUNDATION:

1. FOUNDATION AND FOOTING SIZES ARE BASED ON AN ASSUMED MAXIMUM ALLOWABLE SOIL BEARING PRESSURE OF 1,500 P.S.F. VERIFY LOCAL CONDITIONS AND NOTIFY OWNER OF ANY DISCREPANCY.
2. ENTIRE GROUND SURFACE OF CRAWL SPACE WILL BE COVERED WITH A CLASS 1 VAPOR RETARDER, LAPPED NOT LESS THAN 12 INCHES AT THE JOINTS AND EXTENDED NOT LESS THAN 12 INCHES UP PERIMETER FOUNDATION WALLS.
3. VENTILATION OPENINGS AT PERIMETER FOUNDATION WALLS WILL BE PROVIDED AT A MINIMUM OF 1 SQ. FT. PER 150 SQ. FT. OF UNDER-FLOOR SPACE AREA. VENTILATION OPENINGS WILL BE PLACED TO PROVIDE CROSS VENTILATION. ONE VENTILATION OPENING WILL BE WITHIN 3 FEET OF EACH BUILDING CORNER.
4. VENTILATION OPENINGS SHALL BE PERMITTED TO BE OMITTED ON ONE SIDE.
5. PROVIDE CRAWL SPACE DRAIN AND SLOPE TO LOW POINT FOR POSITIVE DRAINAGE.
6. FOUNDATION WALLS TO HAVE ½" AIR SPACES AT SIDES AND ENDS OF FLOOR BEAMS.
7. SPREAD FOOTINGS WILL NOT BE LESS THAN 6" THICK FOR ONE-STORY WALLS, 7" THICK FOR TWO-STORY WALLS, OR 8" THICK FOR THREE-STORY WALLS.

MINIMUM WIDTH OF CONC. FOOTINGS (INCHES)

CONVENTIONAL LIGHT-FRAME CONSTRUCTION	LOAD-BEARING VALUE OF SOIL (psf)			
	1,000	1,500	2,000	3,000+
1 STORY	15	12	12	12
2 STORY	23	15	12	12
3 STORY	35	23	17	12

MINIMUM HORIZONTAL REINFORCEMENT FOR CONCRETE FOUNDATION WALLS (INCHES)

MAXIMUM UNSUPPORTED HEIGHT OF FOUNDATION WALL (FEET)	LOCATION OF HORIZONTAL REINFORCEMENT
5	ONE NO. 4 BAR WITHIN 12" OF TOP OF WALL
5-8	ONE NO. 4 BAR WITHIN 12" OF TOP OF WALL AND ONE NO. 4 BAR NEAR MID-HEIGHT OF WALL
8+	ONE NO. 4 BAR WITHIN 12" OF TOP OF WALL AND ONE NO. 4 BAR NEAR THIRD POINTS OF WALL STORY

CONCRETE

1. CONCRETE FORMS, SHORING AND POURING METHODS WILL CONFORM TO CURRENT A.C.I. STANDARDS.
2. BACKFILL SHALL NOT BE PLACES AGAINST BASEMENT WALL UNTIL:
   A. CONCRETE OR MASONRY HAS REACHED ITS REQUIRED STRENGTH.
   B. STRUCTURAL FLOOR FRAMING AND SUBFLOORING HAS BEEN COMPLETED TO STABILIZE CONCRETE FOUNDATION WALLS.
3. ALL CONCRETE FOUNDATION WALLS, FOOTINGS AND SLABS SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH AS SPECIFIED PER ONE AND TWO-FAMILY DWELLING CODE.

MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF CONCRETE

TYPE OR LOCATION OF CONCRETE CONSTRUCTION	MINIMUM SPECIFIED COMPRESSIVE STRENGTH (f'c)[1] WEATHERING POTENTIAL[2] NEGLIGIBLE	MODERATE	SEVERE
BASEMENT WALLS AND FOUNDATIONS NOT EXPOSED TO THE WEATHER	2,500	2,500	3,000
BASEMENT SLABS AND INTERIOR SLABS ON GRADE EXCEPT GARAGE FLOOR SLABS	2,500	2,500	3,000
BASEMENT WALLS, FOUNDATION WALLS, EXTERIOR WALLS, AND OTHER VERTICAL CONCRETE WORK EXPOSED TO THE WEATHER	2,500	3,000	3,500
PORCHES, CARPORT SLABS AND STEPS EXPOSED TO THE WEATHER, AND GARAGE FLOOR SLABS	2,500	3,500	3,500

[1] AT 28 DAYS PSI
[2] SEE TABLE R-301.2 FOR WEATHERING POTENTIAL
[3] CONCRETE IN THESE LOCATIONS WHICH MAY BE SUBJECT TO FREEZING AND THAWING DURING CONSTRUCTION SHALL BE ENTRAINED CONCRETE IN ACCORDANCE WITH FOOTNOTE 4.

REINFORCING STEEL

1. ALL REINFORCING STEEL SHALL BE DEFORMED STEEL BARS CONFORMING TO ASTM A615, GRADE 60 FOR SDC 1.
2. ALL REINFORCING STEEL SHALL BE MANUFACTURED, DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 318R, ACI 315R AND ACI SP66.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 IN AS LONG A LENGTH AS IS PRACTICABLE. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST ONE GRID WIDTH PLUS 3".
4. REINFORCEMENT SHALL BE BENT COLD AND SHALL NOT BE WELDED.

   SPLICES:

   REINFORCEMENT IN CONCRETE AND MASONRY SHALL HAVE LAP LENGTHS AS FOLLOWS, UNLESS OTHER SPECIFIED ON DRAWINGS:

BAR SIZES	IN CONCRETE	IN MASONRY
#3	1'-6"	2'-0"
#4	2'-0"	2'-6"
#5	2'-6"	3'-0"

   PLACEMENT:

   A. REINFORCEMENT SHALL BE ACCURATELY PLACES AND SUPPORTED BY CONCRETE, METAL OR OTHER APPROVED CHAIRS, SPACERS OR TIES, AND SECURED AGAINST DISPLACEMENT DURING CONCRETE OR GROUT PLACEMENT.
   B. EXCEPT WHERE OTHERWISE NOTED, REINFORCEMENT SHALL HAVE CONCRETE COVERS AS FOLLOWS:

   CONDITIONS:

1. CONCRETE DEPOSITED AGAINST EARTH	3"
2. FORMED CONCRETE AGAINST EARTH	2"
3. EXTERIOR FACES OF WALLS	2"
4. INTERIOR FACES OF WALLS	3/4"
5. TO TOP OF SLAB-ON-GRADE	3/4"

5. ANCHOR BOLTS IN CONCRETE SHALL CONFORM TO ASTM A307.

CRIPPLE WALLS

1. WHERE INSTALLED (PER PLAN) FOUNDATION CRIPPLE WALLS WILL BE FRAMED OF STUDS NOT SMALLER THAN THE STUDS ABOVE. WHERE CRIPPLE WALLS EXCEED 4 FEET IN HEIGHT, SUCH WALLS WILL BE FRAMED OF STUDS HAVING THE SIZE REQUIRED FOR AN ADDITIONAL STORY.
2. EXTERIOR CRIPPLE WALLS WITH A STUD HEIGHT LESS THAN 14 INCLUES SHALL BE CONTINUOUSLY SHEATHED ON ONE SIDE WITH WOOD STRUCTURAL PANELS FASTENED TO BOTH THE TOP AND BOTTOM PLATES, OR THE CRIPPLE WALLS SHALL BE CONSTRUCTED OF SOLID BLOCKING.
3. CRIPLLE WALLS WILL BE SUPPORTED ON CONTINUOUS FOUNDATIONS.

GLASS AND GLAZING

1. ALL WINDOWS AND PATIO DOORS TO HAVE INSULATED GLAZING.
2. BEDROOM WINDOWS HAVE WINDOW EGRESS OF 5.7. SQ. FT. MINIMUM OPENING NO MORE THAN 44" ABOVE FINISHED FLOOR AND BE A MINIMUM OF 24" IN HEIGHT AND 20" IN WIDTH.
3. ALL WINDOWS 18" FROM FLOOR AND 24" FROM DOORS ARE TO BE TEMPERED GLAZING.
4. ALL TUB AND SHOWER GLASS ENCLOSURES ARE TO BE SAFETY GLAZING.

ENERGY EFFICIENCY

1. INSULATION TO BE INSTALLED WITH VAPOR BARRIER ON INTERIOR FACE.
2. IT MAY BE NECESSARY TO INCREASE THE DEPTH OF FRAMING MEMBERS GREATER THAN SHOWN ON THE DRAWINGS TO ACCOMMODATE THICKER INSULATION MATERIALS, AS MAY BE REQUIRED BY LOCAL CODES OR CONVENTIONS.
3. CAULKING OR GASKETING IS TO BE APPLIED AT ALL EXTERIOR SILLS AND PLATES AND ALL EXTERIOR ENVELOPE PENETRATIONS (BETWEEN WINDOW OR DOOR FRAMES AND ROUGH-OPENINGS).
4. ALL CONDITIONED SPACES WITHIN RESIDENTIAL BUILDINGS WILL COMPLY WITH THE MINIMUM INSULATION PERFORMANCE VALUES BELOW AND FEATURE AS AN ADDITIONAL MEASURE A HIGH-EFFICIENCY GAS-FIRED FURNACE AFUE 94 PERCENT OR BETTER.

MINIMUM INSULATION PERFORMANCE VALUES:

1. CEILINGS	
A. VAULTED AREAS:	R-30
B. FLAT CEILINGS:	R-49
2. EXTERIOR WALLS (2X6 STUDS):	R-23
3. FLOORS OVER UNHEATED SPACE:	R-30
4. BASEMENT WALLS:	
A. CONTINUOUS INSULATION:	R-15
B. FRAMED CAVITIES:	R-21
1. SLAB FLOOR-ON-GRADE:	R-15
2. FORCED AIR DUCTS:	R-8
3. WINDOW GLASS:	U=0.27
4. EXTERIOR DOORS:	U=0.20
A. ONE DOOR, MAX 28 FT²	U=0.54
5. EXTERIOR DOORS WITH 2.5 FT² OF GLAZING:	U=0.40

5. VOIDS IN HEADERS 1-2 INCHES IN THICKNESS WILL BE INSULATED WITH INSULATION HAVING A VALUE OF R-4 OR GREATER PER INCH. VOIDS IN HEADERS GREATER THAN 2 INCHES IN DEPTH WILL BE INSULATED TO A MINIMUM OF R-10. NON-STRUCTURAL HEADERS WILL BE REPLACED WITH INSULATION TO ACHIEVE THERMAL PERFORMANCE EQUIVALENT TO THE ADJACENT AREA.
6. A CONTINUOUS AIR BARRIER WILL BE INSTALLED AND FULLY ALIGNED WITH THE BUILDING THERMAL ENVELOPE ON EVERY VERTICAL PORTION OF AIR-PERMEABLE INSULATION AND ON THE WARM SIDE OF HORIZONTAL, AIR-PERMEABLE INSULATION.
7. ALL TOP PLATES IN CONTACT WITH VENTED ATTICS WILL BE SEALED WITH APPROVED MATERIAL.
8. ALL DUCT SYSTEMS AND AIR HANDLING EQUIPMENT WILL BE LOCATED FULLY WITHIN THE BUILDING THERMAL ENVELOPE, WITH THE FOLLOWING EXCEPTIONS:
   A. UP TO 5 PERCENT OF THE LENGTH OF AN HVAC SYSTEM MAY BE PERMITTED TO BE OUTSIDE THE THERMAL ENVELOPE.
   B. DUCTS DEEPLY BURIED IN INSULATION, WHERE INSULATION IS INSTALLED TO FILL GAPS BETWEEN THE DUCT AND CEILING AND A MINIMUM OF R-19 INSULATION SHALL BE INSTALLED ABOVE THE DUCT AND INSULATION DEPTH MARKER FLAGS ARE INSTALLED EVERY 10 FEET OR AS APPROVED.
9. ALL BATHROOM EXHAUST FANS WILL BE ENERGY STAR CERTIFIED.
10. ALL CENTRAL FURNACES WILL HAVE ELECTROSTATICALLY COMMUTATATED FAN MOTORS WITH A FAN EFFICIENCY RATING MEETING 10 CFR 430.32(Y).
11. ALL PERMANENT LIGHTING FIXTURES WILL BE HIGH EFFICIENCY LIGHT SOURCES, WITH THE EXCEPTION OF UP TO TWO FIXTURES THAT ARE CONTROLLED BY A DIMMER OR AUTOMATIC CONTROL.

MISCELLANEOUS

1. PROVIDE COMBUSTIBLE AIR INLET FOR FIREPLACE(S) AND WOODSTOVE(S).
2. TUB AND SHOWERS TO HAVE ½" WATER PROOF SHEETROCK WALLS TO HAVE HARD MOISTURE RESISTANT SURFACE UP TO 6'-0" MINIMUM.
3. EXHAUST FANS, RANGE HOODS, CLOTHES DRYERS SHALL BE VENTED TO EXTERIOR.
4. ALL INTERIOR ROOM DIMENSIONS ARE APPROXIMATE ESTIMATES. DUE TO UNCONTROLLABLE CONSTRUCTION METHODS, SOME DIMENSIONS ARE SUBJECT TO CHANGE.
5. SMOKE DETECTORS TO BE CONNECTED TO HOUSE POWER. A SERIES OF DETECTORS MUST BE INTERCONNECTED, BOTH UP AND DOWN STAIRS AND IN EVERY BEDROOM.
6. LIGHTING SHOWN ON PLANS IS INTENDED FOR USE ONLY AS A GUIDE. ELECTRICAL SWITCHES, OUTLETS AND PLUG-INS ARE TO BE INSTALLED PER LOCAL CODES AND THE OWNER/BUILDER'S REQUIREMENTS.
7. EXTERIOR AND INTERIOR WALLS ARE INCLUDED IN TOTAL SQUARE FOOTAGE.
8. ANY ROOM WITH A BATHTUB OR SHOWER OR TOILER WILL BE PROVIDED WITH A MECHANICAL VENTILATION DEVICE CONTROLLED BY DEHUMIDISTAT, TIMER OR SIMILAR MEANS OF AUTOMATIC CONTROL.

RADON MITIGATION

MANDATORY FOR ALL NEW CONSTRUCTION IN BAKER, CLACKAMAS, HOOD RIVER, MULTNOMAH, POLK, WASHINGTON AND YAMHILL COUNTIES.

THE FOLLOWING RADON CRAWL SPACE MITIGATION MEASURES WILL BE USED IN ACCORDANCE WITH 2021 ORSC SECTION AF103(2), RADON-RESISTANT CONSTRUCTION FOR CRAWL SPACE FOUNDATIONS.

1. CRAWL SPACES WILL BE PROVIDED WITH VENTS TO THE EXTERIOR OF THE BUILDING. THE MINIMUM NET AREA OF VENTILATION OPENINGS WILL COMPLY WITH SECTION R408.1.
2. A UNIFORM LAYER OF GAS-PERMEABLE AGGREGATE GRAVEL NO LESS THAN 4-INCHES THICK (WITH AGGREGATE DIAMETER BETWEEN 1/4"-2") WILL BE LAID DIRECTLY ATOP SOIL BELOW CRAWL SPACE FLOOR SYSTEMS.
3. THE AGGREGATE LAYER IN CRAWL SPACES WILL BE COVERED WITH A CONTINUOUS LAYER OF 6-MIL POLYETHYLENE SOIL-GAS RETARDER. THE GROUND COVER WILL BE LAPPED NO LESS THAN 12" AT JOINTS AND WILL EXTEND TO ALL FOUNDATION WALLS ENCLOSING THE CRAWL SPACE.
4. OPENINGS AROUND PLUMBING, WIRES OR OTHER PENETRATIONS THROUGH THE FLOOR ASSEMBLY WILL BE FILLED WITH POLYURETHANE CAULK OR OTHER APPROVED SEALANT.
5. EXTERIOR SURFACES OF CONCRETE WALLS BELOW GRADE WILL BE DAMPPROOFED IN ACCORDANCE WITH SECTION R406.
6. CRAWL SPACE ACCESS OPENINGS THROUGH FLOOR ASSEMBLIES WILL BE CLOSED, GASKETING OR OTHERWISE SEALED TO PREVENT AIR LEAKAGE.

FURTHERMORE, FOR ALL BUILDINGS WITHOUT AN APPROVED MECHANICAL CRAWL SPACE VENTILATION SYSTEM, THE FOLLOWING RADON MITIGATION MEASURES WILL BE INSTALLED:

AF103.3.1 PASSIVE SUBMEMBRANE DEPRESSURIZATION SYSTEM

7. A PLUMBING TEE OR OTHER APPROVED CONNECTION WILL BE INSERTED HORIZONTALLY BENEATH THE SHEETING AND CONNECTED TO A 3- OR 4-INCH DIAMETER FITTING WITH A VERTICAL VENT PIPE INSTALLED THROUGH THE SHEETING. THE VENT PIPE WILL BE EXTENDED UP THROUGH THE BUILDING FLOORS AND TERMINATE NOT LESS THAN 12 INCHES ABOVE THE ROOF IN A LOCATION NOT LESS THAN 10 FEET AWAY FROM ANY WINDOW OR OTHER OPENING INTO THE CONDITIONED SPACES OF THE BUILDING THAT IS LESS THAN 2 FEET BELOW THE EXHAUST POINT, AND 10 FEET FROM ANY WINDOW OR OTHER OPENING IN ADJOINING OR ADJACENT BUILDINGS.

STONE BRIDGE HOMES NW

PLAN No.	STANDARD
DRAWN BY:	MG
DATE:	10/1/21
SCALE:	N.T.S.
FILE:	GNOTE

DRAWINGS:

GENERAL NOTES AND REQUIREMENTS

SHEET No.

GN

GENERAL REQUIREMENTS

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

DESCRIPTION OF BUILDING MATERIALS	NUMBER & TYPE OF FASTENER a,b,c,e	SPACING OF FASTENERS	
Joist to sill or girder, toe nail.	3- 8d	-	
1"x6" subfloor or less to each joist, face nail.	2- 8d 2 staples, 1 3/4"	- -	
Wider than 1"x6" subfloor to each joist, face nail.	3- 8d 4 staples, 1 3/4"	- -	
2" subfloor to joist or girder, blind and face nail.	2- 16d	-	
Sole plate to joist or blocking face nail.	16d	16" o.c.	
Top or sole plate to stud, end nail.	2- 16d	-	
Stud to sole plate, toe nail.	3- 8d or 2- 16d	-	
Double studs, face nail.	10d	24" o.c.	
Double top plate, face nail.	10d	24" o.c.	
Dbl. top plates, min. 24" offset of end joints, face nail in lapped area	8- 16d [J]	-	
Continued header, two pieces.	16d	16" o.c. along each edge	
Ceiling joists to plate, toe nail.	3- 8d	-	
Continuous header to stud, toe nail.	4- 8d	-	
Ceiling joist not to parallel rafters, laps over partitions, face nail.	3- 10d	-	
Rafter to plate, toe nail.	2- 16d	-	
1" brace to each stud and plate, face nail.	2- 8d 2 staples, 1 3/4"	- -	
1"x6" sheathing to each bearing, face nail.	2- 8d 2 staples, 1 3/4"	- -	
1"x8" sheathing to each bearing, face nail.	2- 8d 3 staples, 1 3/4"	- -	
Wider than 1"x8" sheathing to each bearing, face nail.	3- 8d 4 staples, 1 3/4"	- -	
Built-up corner studs.	10d	24" o.c.	
Built-up girder and beams.	10d	32"o.c. at top and bottom and staggered 2- 20d at ends and each splice.	
2" planks.	2- 16d	At each bearing	
Roof rafters to ridge, valley, or hip rafters, toe nail, face nail.	 4- 16d 3- 16d	 - -	
Plywood and particleboard, roof and wall sheathing to frame.		**Edges** [i]	**Int. supports** [c, e]
3/8" - 1/2"	6d (subfloor wall) 8d (roof) [f]	6"	12"
19/32" - 1"	8d	6"	12"
1 1/8" - 1 1/4"	10d smooth or 8d deformed	6"	12"
Other wall sheathing. [h]			
1/2" fiberboard sheathing	1 1/2" ga. roofing nail 7/16" crown or 1" crown staple 16 ga. 1 1/4" long	3"	6"
25/32" fiberboard sheathing	1 3/4" ga. roofing nail 1/16" crown or 1" crown staple 16 ga. 1 1/2" long	3"	6"
1/2" gypsum sheathing [d]	1 1/2" ga. roofing nail staple 16 ga. 1 1/2" long 1 1/4 screws, type W or S	7"	7"
5/8" gypsum sheathing [d]	1 3/4" ga. roofing nail staple 16 ga. 1 5/8" long 1 5/8 screws, type W or S	7"	7"
Plywood and particleboard, combination subfloor-underlayment to framing.			
3/4" and less	6d deformed or 8d common	6"	12"
7/8" - 1"	8d common or 8d deformed	6"	12"
1 1/8" - 1 1/4"	10d common or 8d deformed	6"	12"

All nails are smooth-common, box, or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi for shank dia. of 0.192" (20d common nail), 90 ksi for shank dia. larger than 0.142 " but not larger than 0.177", and 100 ksi for shank dia. of

Staples are No. 16 guage wire and have a minimum 7/16 inch O.D.

Nails shall be spaced at not more than 6 inch o.c. at all supports where spans are 48 inches or greater.

Four-foot by 8-foot or 4-foot by 9-foot panels shall be

Spacing of fasteners not included in this table shall be based on ORSC Table No. R602.3(2).

For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within min. 48" distance from gable end walls, if mean roof ht. is more than 25 feet, up

For regions having basic wind speed of less than 110 mph, nails for attaching wood structural panel roof sheating to gable end wall framing shall be spaced 6" o.c. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6" o.c. for min. 48" distance from ridges, eaves and gable end walls± and 4" o.c. to gable end

Gypsum sheathing shall conform to ASTM C1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C208. Other sheathing shall be approved by the building official.

Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and required blocking and at all floor perimeters only. Spacing of fasteners on roof sheating panel edges applies to panel edges supported by framing members and required blocking. Blocking of roof or floor sheathing panel edges perp. to the framing members need not be provided except as req'd. by other provisions of this code. Floor perim. shall be supported by framing members or solid blocking.

Interior non-braced wall lines may be nailed with a min. (4) 10d nails.

STONE BRIDGE HOMES NW

PLAN No. STANDARD
DRAWN BY: MG
DATE: 10/1/21
SCALE: N.T.S.
FILE: GNOTE
DRAWINGS:

GENERAL NOTES AND REQUIREMENTS

SHEET No.

GN2

STONE BRIDGE HOMES NW

4230 GALEWOOD ST. SUITE 100
LAKE OSWEGO, OR 97035
(503) 387-7577

ZONING: R-7
MAX. HEIGHT: 35'-0"
FRONT SETBACK: 14'-0"
GARAGE SETBACK: 20'-0"
REAR SETBACK: 15'-0"
SIDE SETBACK: 5'-0"
STREET SIDE SETBACK: 11'-0"

OBE:3150

LOT: 15
DATE: REV 3/09/2023
PROPERTY: FARMSTEAD CROSSING
CITY: FOREST GROVE
SCALE: 1"=20'
PLAN No.: 300
ELEVATION: STANDARD

LOT COVERAGE

LOT AREA:	5,176 SQ. FT.
BUILDING AREA:	2,425 SQ. FT.
PERCENTAGE:	46.85 %

TREE LEGEND

CARPINUS JAPONICA
2" CALIPER

BUILDING AREA

MAIN:	1,858 SQ. FT.
UPPER:	- SQ. FT.
GARAGE:	567 SQ. FT.
PORCH:	80 SQ. FT.
COV. PATIO:	144 SQ. FT.

LOT 16

APPROVED
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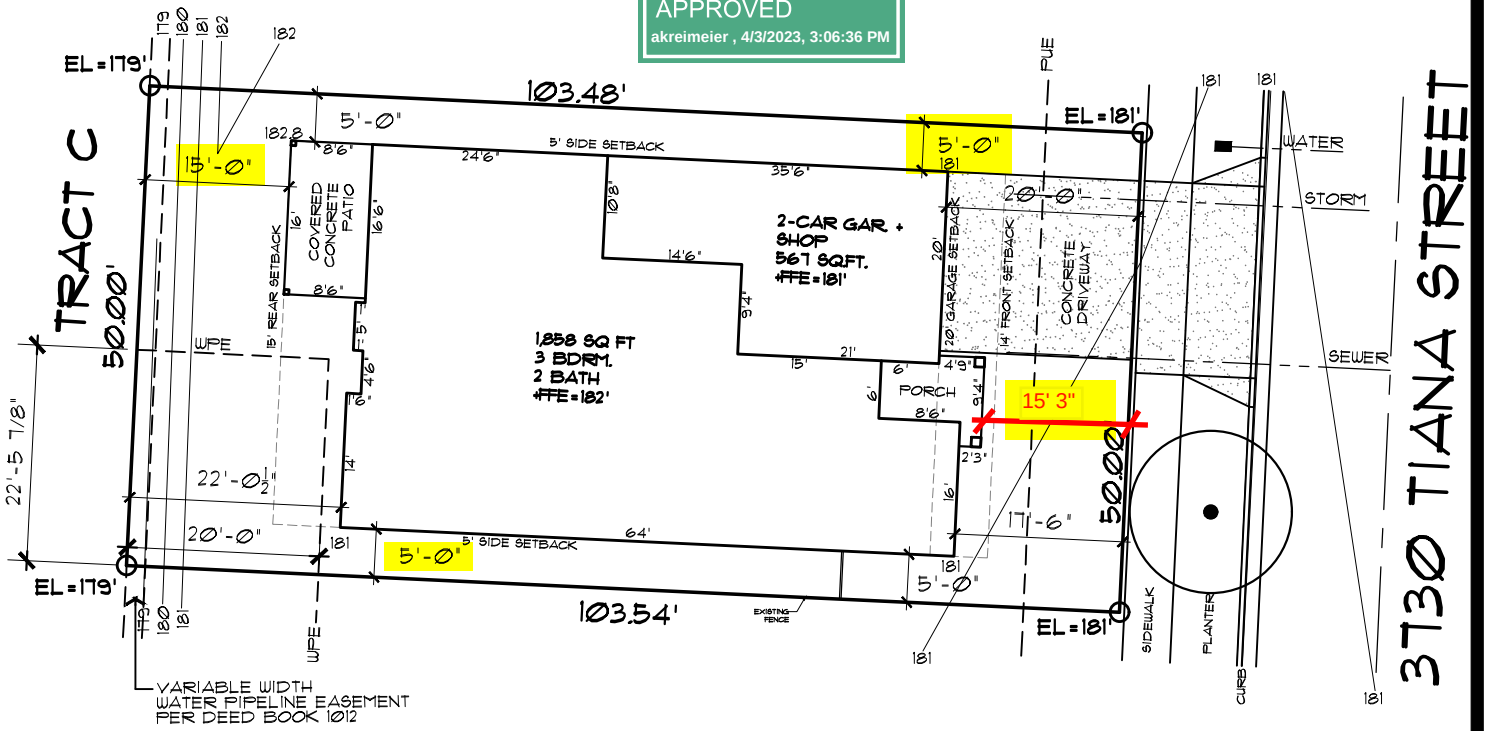


LOT 14

NOTES:

ALL GRADE AND PROPERTY LINES ARE ESTIMATES OF CURRENT LOCATIONS.
ALL DIMENSIONS AND SQUARE FOOTAGE ARE APPROXIMATE FIGURES.
ALL RETAINING WALL HEIGHTS AND LOCATIONS ARE ESTIMATES.
THEY MAY VARY AND BE SUBJECT TO CHANGE.

LOT #15
5,176 SQ. FT.

FOREST GROVE OREGON

A place where families and businesses thrive.

J3150
FC15

RECEIVED
4/3/23

Building Permit Application

City of Forest Grove

Permit Number: 311-23-000206-DWL

TYPE OF WORK	
☑ New construction	☐ Demolition
☐ Addition/alteration/replacement	☐ Other:

CATEGORY OF CONSTRUCTION	
☑ 1- and 2-family dwelling	☐ Commercial/industrial
☐ Accessory building	☐ Multi-family
☐ Master builder	☐ Other:

JOB SITE INFORMATION AND LOCATION

Job site address: 3730 Tiana St.

City/State/ZIP: Forest Grove, OR 97116

Suite/bldg./apt. no.: | Project name: Farmstead Crossing

Cross street/directions to job site:

Subdivision: Farmstead Crossing | Lot no.: 15

Tax map/parcel no.:

DESCRIPTION OF WORK

New, single family residence

☑ PROPERTY OWNER	☐ TENANT

Name: Stone Bridge Homes NW, LLC

Address: 4230 Galewood St. Suite #100

City/State/ZIP: Lake Oswego, OR 97035

Phone: 503-387-7577 | Email: portlandpermits@stonebridgehomesnw.com

☑ APPLICANT	☐ ENGINEER	☐ ARCHITECT/DESIGNER

Business name: Stone Bridge Homes NW, LLC

Contact name: Permit Tech

Address: 4230 Galewood St. Suite #100

City/State/ZIP: Lake Oswego, OR 97035

Phone: 503-387-7577 | Email: portlandpermits@stonebridgehomesnw.com

CONTRACTOR

Business name: Stone Bridge Homes NW, LLC

Address: 4230 Galewood St. Suite #100

City/State/ZIP: Lake Oswego, OR 97035

Phone: 503-387-7577 | Email: portlandpermits@stonebridgehomesnw.com

CCB lic.: 173318 | City/Metro Bus lic:

Authorized signature: Tiana Rudolf

Print name: Tiana Rudolf | Date: 3-27-2023

REQUIRED DATA: 1- AND 2-FAMILY DWELLING		
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.		
Valuation	209,857	$321,953.95
Number of bedrooms:	3	
Number of bathrooms:	2	
Total number of floors:	1	
New dwelling area:	1,858	square feet
Garage/carport area:	567	square feet
Covered porch area:	244	square feet
Deck area:		square feet
Other structure area:		square feet

REQUIRED DATA: COMMERCIAL-USE CHECKLIST	
Permit fees* are based on the value of the work performed. Indicate the value (rounded to the nearest dollar) of all equipment, materials, labor, overhead, and the profit for the work indicated on this application.	
Valuation	
Existing building area:	square feet
New building area:	square feet
Number of stories:	
Type of construction:	
Occupancy groups:	
Existing:	
New:	

NOTICE

All contractors and subcontractors are required to be licensed with the Oregon Construction Contractors Board under ORS 701 and may be required to be licensed in the jurisdiction in which work is being performed. If the applicant is exempt from licensing, the following reasons apply:

BUILDING PERMIT FEES*	
Please refer to fee schedule	
Fees due upon application	
Amount received	
Date received	

This permit application expires if a permit is not obtained within 180 days after it has been accepted as complete

* Fee methodology set by Tri-County Building Industry Service Board

1924 Co

LIGHT & POWER
CC 4/6/23

City of Forest Grove – Light & Power Review
Single Family Residential

3730 Tiana St **Permit #** 311-22-000206-DWL

1. Builder to locate meter base on NORTH wall of GARAGE within 3 feet of

The front corner of the structure, or as follows:

2. For underground service up to 200 amps, builder to provide and install 3" Sch. 40 PVC conduit (at 36" minimum depth with a maximum of 3 – 90 degree bends) from the meter base to the pedestal/pole located at:

SOUTHEAST EDGE OF PROPERTY.
SEE ELECTRICAL SITE REVIEW...

Light & Power Dept. to inspect all conduit installations prior to backfilling. (503) 992-3250

3. Utility provided temporary construction service, including permit, is available for a $225.00 fee. In addition, line extension fees are charged if the cost to connect to the Utility System exceeds a pre-set allowance. Contact the Light & Power Dept. at (503) 992-3250.

Comments: Light & Power can provide and install the duct run for an underground service for an additional cost. Contact Light & Power for information. (503) 992-3294
Note: If service is to be over 200 amps, Light & Power must be contacted prior to installation.

Additional comments: The standard cost of a service line extension for a single family residence is $450. Unusual circumstances may require additional charges.

IF THE SERVICE CANNOT BE INSTALLED AS INDICATED, CONTACT
JASON O'DELL FOR ALTERNATIVE OPTIONS. (503)992-3294

CITY OF FOREST GROVE
LIGHT & POWER DEPT.
1818 "B" STREET
P.O. BOX 326
FOREST GROVE, OR 97116
- REVIEW -
CC - 4/10/23

- ELECTRICAL SITE PLAN -

ONE BRIDGE
HOMES NW
4230 GALEWOOD ST. SUITE 100
LAKE OSWEGO, OR 97035
(503) 387-7577

ZONING: R-1
MAX. HEIGHT: 35'-0"
FRONT SETBACK: 14'-0"
GARAGE SETBACK: 20'-0"
REAR SETBACK: 15'-0"
SIDE SETBACK: 5'-0"
STREET SIDE SETBACK: 11'-0"

OBE:3150
LOT: 15
DATE: REV 3/09/202
PROPERTY: FARMSTEA
CROSSING
CITY: FOREST GROVE
SCALE: 1"=20'
PLAN No.: 300
ELEVATION: STANDARI

3730 Tiana St

LOT COVERAGE

LOT AREA:	5,176 SQ. FT.
BUILDING AREA:	2,425 SQ. FT.
PERCENTAGE:	46.85 %

BUILDING AREA

MAIN:	1,858 SQ.
UPPER:	- SQ.
GARAGE:	567 SQ
PORCH:	80 SQ. FT.
COV. PATIO:	144 SQ. FT.

TREE LI

- IMPORTANT NOTE -
A MINIMUM OF 8 FEET OF CLEARANCE IS REQUIRED BETWEEN THE PAD MOUNT TRANSFORMER & THE STRUCTURE/HOUSE, INCLUDING THE ROOF OVERHANG. (SEE ATTACHMENT, OR PAGE 5 OF THE FGL&P DEPT. "SERVICE REQUIREMENTS & GUIDELINES" HANDBOOK). BUILDER WILL NEED TO VERIFY CLEARANCE & MODIFY STRUCTURE IF NEEDED TO MEET THE REQUIRED CLEARANCE.

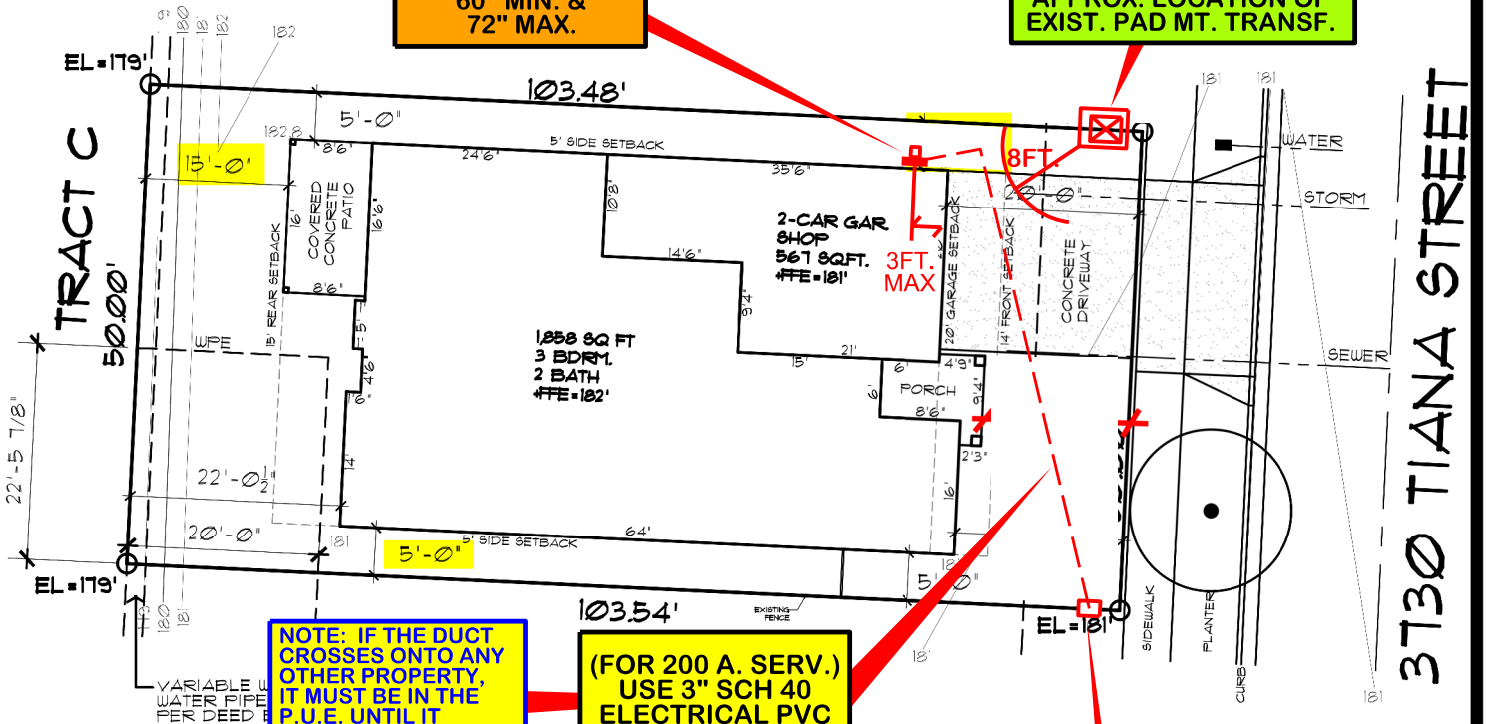
ANGLE 90° BEND AT METER BASE END TOWARD FRONT OF LOT, AS INDICATED

NOTE: L&P TO CHECK TRANSFORMER CLEARANCE AT TIME OF PERMANENT ELECTRIC SERVICE CONNECTION. IF 8FT. MIN. CLEARANCE HAS NOT BEEN MET, THE SERVICE WILL NOT BE ENERGIZED.

ELECTRIC METER BASE LOCATION, HT. FROM FINISHED GRADE 60" MIN. & 72" MAX.

LOT 16

APPROX. LOCATION OF EXIST. PAD MT. TRANSF.



3730 TIANA STREET

NOTE: IF THE DUCT CROSSES ONTO ANY OTHER PROPERTY, IT MUST BE IN THE P.U.E. UNTIL IT CROSSES BACK ONTO LOT 15.

(FOR 200 A. SERV.) USE 3" SCH 40 ELECTRICAL PVC CONDUIT, MIN. DEPTH 36"

APPROX. LOCATION OF EXIST. SECONDARY POWER PEDESTAL

CONNECT 3" DUCT TO 90° SWEEP STUBBED OUT FROM PEDESTAL.

IF DUCT RUN CANNOT BE KEPT BELOW 270° OF BENDS THEN STUB OUT WILL HAVE TO BE DUG BACK TO PED & TRAJECTORY CORRECTED

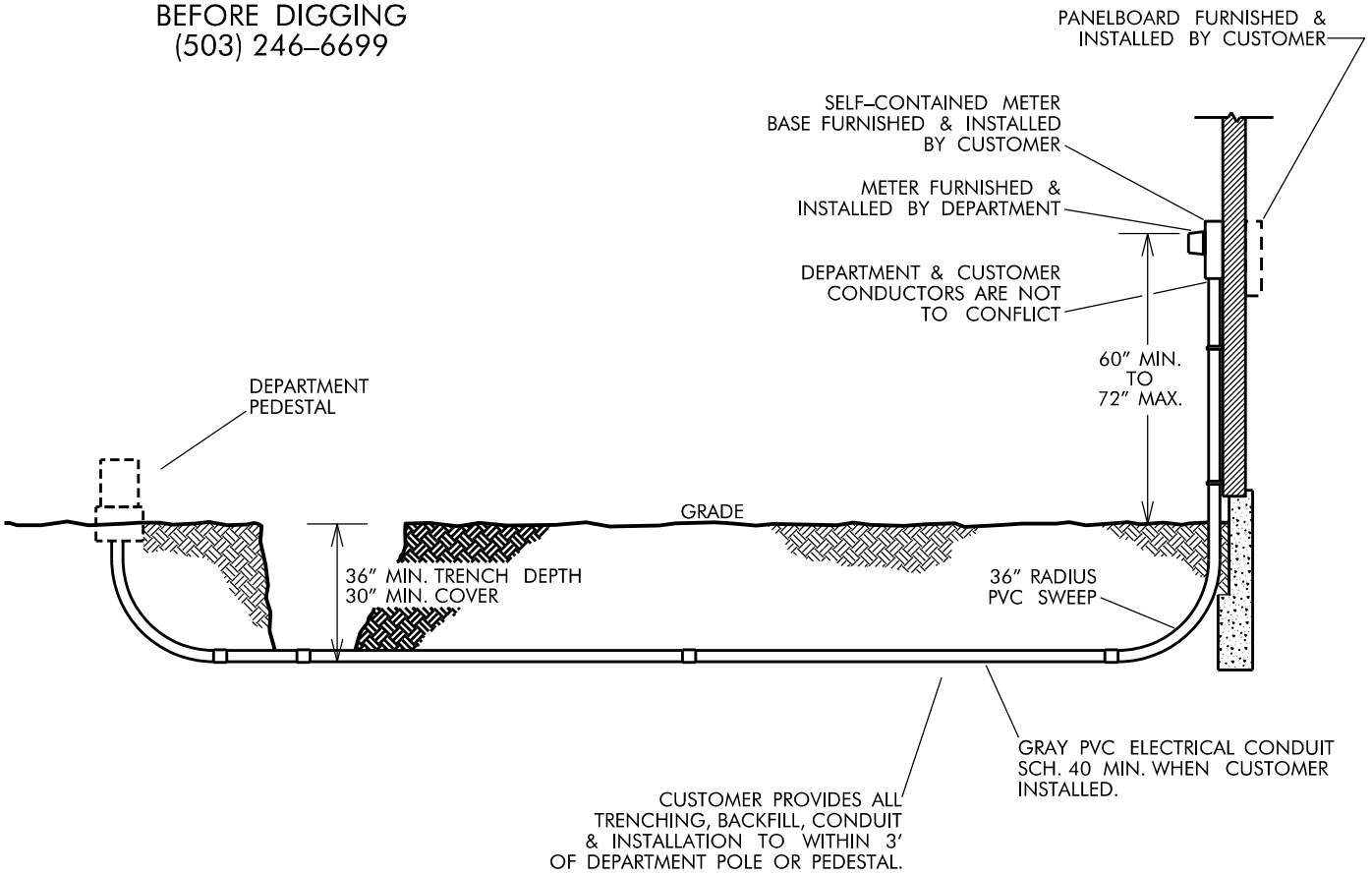
NOTE: A MAXIMUM TOTAL OF ONLY 270° OF BENDS CAN BE USED FOR THE DUCT RUN, INCLUDING THE 90° BEND AT EACH END OF THE DUCT RUN. CONTACT FGL&P IF NEEDED FOR OPTIONS.

NOTE: USE CITY OF FOREST GROVE LIGHT AND POWER DEPARTMENT SPECIFICATIONS, NOT PGE. IF SPECIFICATIONS ARE NOT CLEAR CALL (503) 992-3250

"This design complies with ORS 92.044(7) in that no utility infrastructure is designed to be within 1 ft. of a survey monument location shown on a subdivision or partition plat. No design modification nor final field location change shall be permitted if it would cause any utility infrastructure to be placed within the prohibited area."

LOT #15
5,176 SQ. FT.

ALWAYS CALL
BEFORE DIGGING
(503) 246–6699



TYPICAL UNDERGROUND SERVICE INSTALLATION

NOTES:

① TRENCH & CONDUIT ARE TO BE INSPECTED BY FGL&P PRIOR TO BACKFILLING THE TRENCH. IF MORE THAN 1 SWEEP NEEDS TO BE INSTALLED IN THE CONDUIT RUN, NOT INCLUDING THE SWEEP AT THE POLE OR PEDESTAL (INSTALLED BY FGL&P) OR THE SWEEP AT THE METER BASE, CONTACT FGL&P PRIOR TO EXCAVATING & INSTALLING CONDUIT.

② THE METER BASE SHALL BE LOCATED ON THE FRONT WALL OR WITHIN 3' OF THE FRONT WALL ON EITHER OF THE SIDE WALLS.

SERVICE TRENCH DETAIL SPECIFICATION	FOREST GROVE LIGHT & POWER	
	DATE: 1/19/19	*SCALE:* NTS
	DRAWN BY: MEH	*REVIEWED:* 1/21/11–NL
	REVISED: JO–5/21/19	R5

2.06 METERING EQUIPMENT CLEARANCES

All meters, metering equipment, and enclosures must be readily accessible by Forest Grove Light & Power personnel at all times for meter reading, maintenance, testing, installation, or removal. They must be at a location free from obstructions, vibration, corrosive atmosphere, and abnormal temperatures. Working space in front of meters or metering equipment (including instrument transformer enclosures) must be at least 36 inches measured from the front of the meter glass or enclosure. Working space behind freestanding switchboards must be at least 36 inches from the panel to any obstruction, with provisions for safe exit. Refer to Figure 7 for details. To maintain safe working conditions, no exceptions will be allowed (NEC). A minimum clearance of 36 inches must be maintained between the electric meter enclosure and gas meter.

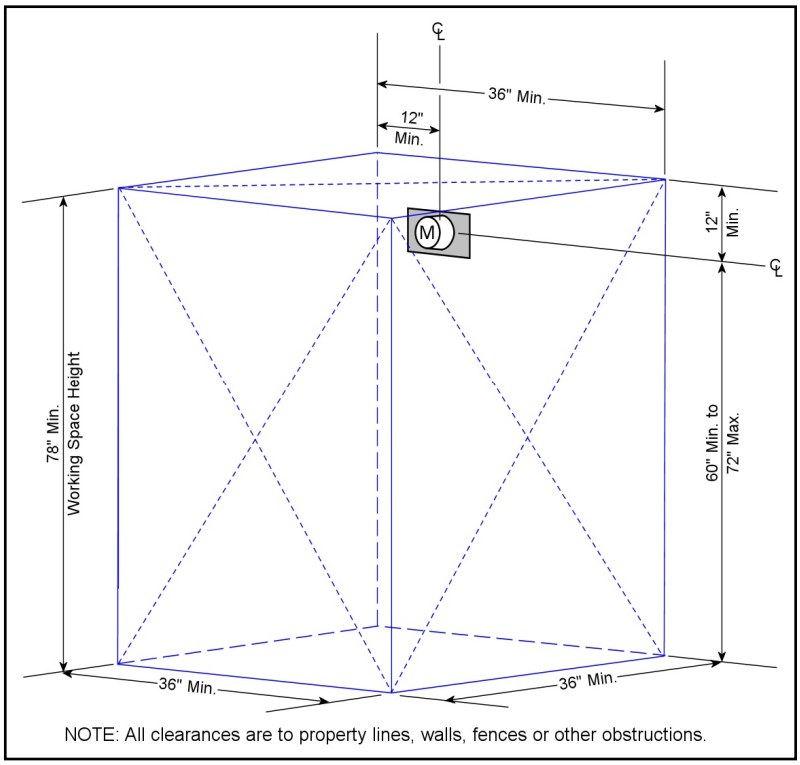


Figure 7: Clearances to Meters

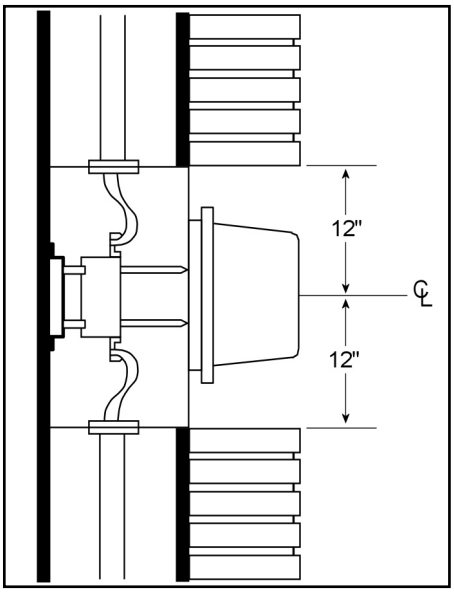


Figure 8: Flush-Mount Meter

The center of all meter socket enclosures must be a minimum of 12 inches from walls, ceilings, service equipment, or other obstructions. Building siding may not cover or overlap any portion of the meter socket. Refer to Figure 8.

1.09 CLEARANCES FROM UTILITY EQUIPMENT

This section covers clearances which must be maintained around Utility equipment. For equipment which is located in areas subject to vehicular traffic, the Customer shall provide and install steel traffic bollards, painted yellow, and set in concrete. If you have a special situation or question, please contact Forest Grove Light & Power at (503) 992-3250.

Poles

In order to ensure the safety of Utility personnel, a clearance of 10 feet from all Utility poles to any structures or equipment must be maintained. Any obstructions found in this clear zone will be removed by the Utility. In addition, nothing may be attached to Utility poles. This includes, but is not limited to, all signs and placards. Attachments will be removed and the violators may be responsible for the cost of removal.

Padmount Equipment

In general, a minimum clearance of three feet is required around the sides and back of padmounted electrical equipment. A separation of eight feet shall be maintained between transformers and building openings. In no event shall any portion of a combustible structure encroach into the clearance zone 8' in any direction from a transformer at ground level. The clearance zone includes all space below and above the 8' ground level clearance zone. For all equipment, the minimum clearance in front is eight feet. No obstructions, including fences, retaining walls, poles, mail boxes, shrubs, trees, or landscaping, may be placed within this area. Refer to Figure 3 for details.

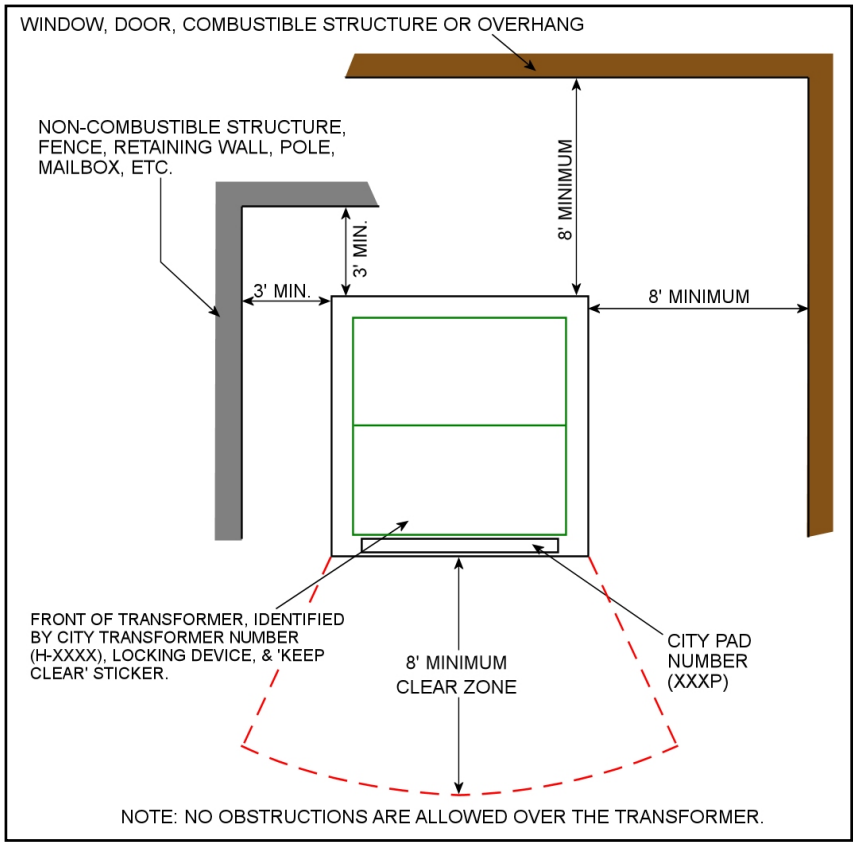
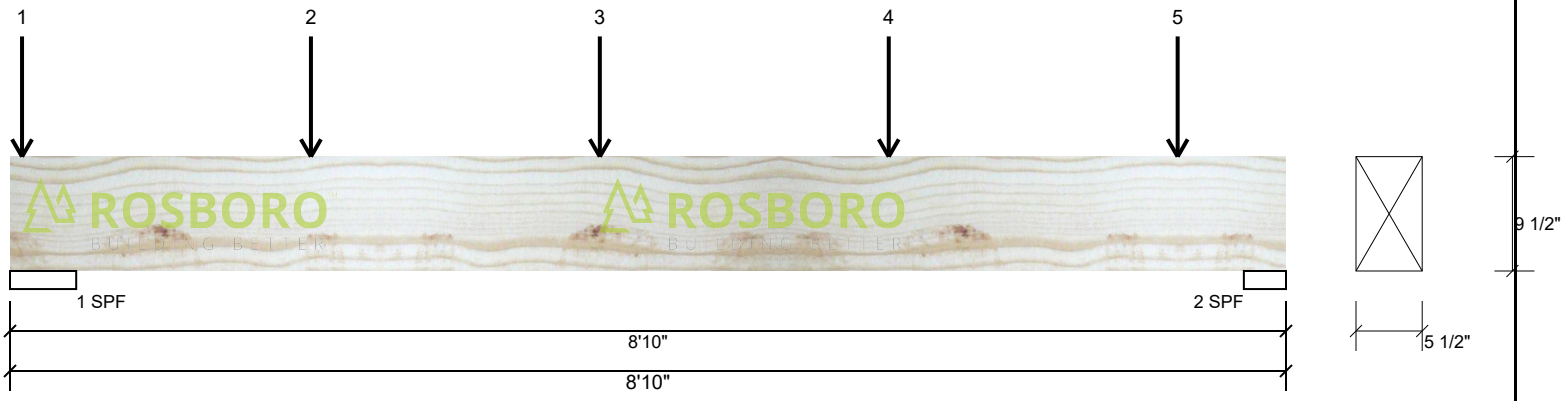


Figure 3: Clearances to Pad Mount Ttransformers (Plan View)

Client:		Date:	3/30/2023
Project:		Input by:	
Address:		Job Name:	3150 FC15 BEAMS
		Project #:	

A Rosboro 24F-V4 5.500" X 9.500" - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Moisture Condition:	Dry	Design Method:	ASD
Deflection LL:	480	Building Code:	IBC/IRC 2015
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Wet Use:	No
Temperature:	Temp <= 100°F	Deck:	Not Checked

Reactions PATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2260	0	0	0
2	Vertical	0	1908	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	Vert	18%	2260 / 0	2260	Uniform	D
2 - SPF	3.500"	Vert	23%	1908 / 0	1908	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3615 ft-lb	4'1"	14891 ft-lb	0.243 (24%)	D	Uniform
Unbraced	3615 ft-lb	4'1"	14820 ft-lb	0.244 (24%)	D	Uniform
Shear	1562 lb	7'9"	8308 lb	0.188 (19%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.060 (L/1633)	4'5 15/16"	0.410 (L/240)	0.147 (15%)	D	Uniform

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Girders are designed to be supported on the bottom edge only.
3 Top must be laterally braced at bearings.
4 Bottom must be laterally braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Point	0-1-0		Top	811 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
2	Point	2-1-0		Top	811 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
3	Point	4-1-0		Top	811 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
4	Point	6-1-0		Top	811 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								

Continued on page 2...

Notes

It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads.

Engineered Wood Products

1. Dry service conditions, unless noted otherwise
2. No treatment with fire-retardant or other strength-reducing chemicals.

Handling & Installation

1. Engineered wood products must not be cut or drilled. Damaged products shall not be used.
2. Refer to the latest version of the installation guide for construction details, hole specifications, multiple-member connections, and handling guidelines.
3. Provide lateral support at bearing points to prevent lateral displacement and rotation.
4. For flat roof, provide proper drainage to prevent ponding.

This design is valid until 5/30/2024

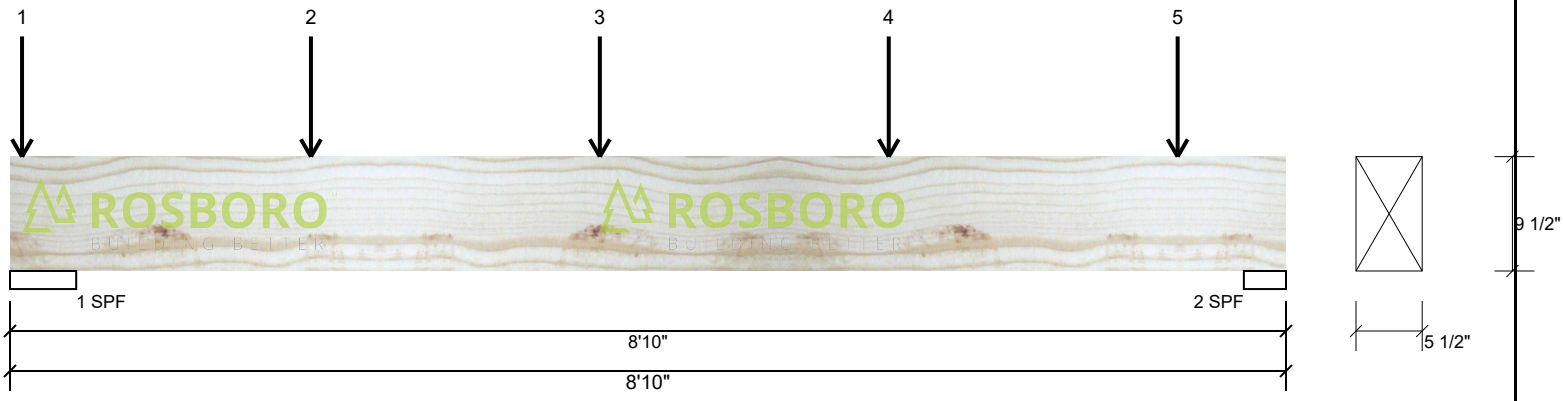
Manufacturer Info

Rosboro
Springfield, OR 97477
(877) 457-4139
www.rosboro.com
APA: PR-L275, ICC-ES: ESR-1940

Client:		Date:	3/30/2023
Project:		Input by:	
Address:		Job Name:	3150 FC15 BEAMS
		Project #:	

A Rosboro 24F-V4 5.500" X 9.500" - PASSED

Level: Level



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
5	Point	8-1-0		Top	811 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
	Self Weight				13 PLF					

Notes

It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads.

Engineered Wood Products

1. Dry service conditions, unless noted otherwise
2. No treatment with fire-retardant or other strength-reducing chemicals.

Handling & Installation

1. Engineered wood products must not be cut or drilled. Damaged products shall not be used.
2. Refer to the latest version of the installation guide for construction details, hole specifications, multiple-member connections, and handling guidelines.
3. Provide lateral support at bearing points to prevent lateral displacement and rotation.
4. For flat roof, provide proper drainage to prevent ponding.

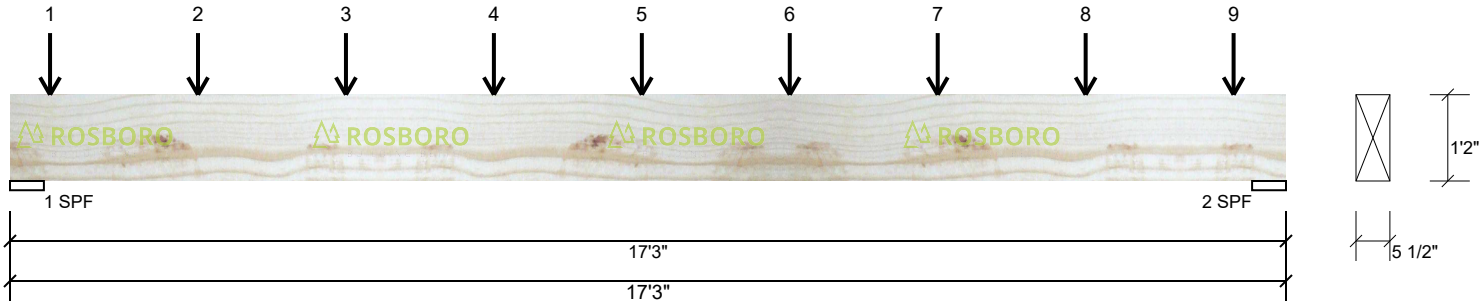
This design is valid until 5/30/2024

Manufacturer Info

Rosboro
Springfield, OR 97477
(877) 457-4139
www.rosboro.com
APA: PR-L275, ICC-ES: ESR-1940

H1 Rosboro 24F-V4 5.500" X 14.000" - PASSED

Level: Level



Member Information

Type:	Header	Application:	Floor
Moisture Condition:	Dry	Design Method:	ASD
Deflection LL:	480	Building Code:	IBC/IRC 2015
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Wet Use:	No
Temperature:	Temp <= 100°F	Header Supports Glass:	No
		Deck:	Not Checked

Reactions PATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	4572	0	0	0
2	Vertical	0	5889	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	Vert	36%	4572 / 0	4572	Uniform	D
2 - SPF	5.500"	Vert	46%	5889 / 0	5889	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	20522 ft-lb	8'6 1/2"	32340 ft-lb	0.635 (63%)	D	Uniform
Unbraced	20522 ft-lb	8'6 1/2"	31818 ft-lb	0.645 (64%)	D	Uniform
Shear	4791 lb	15'7 1/2"	12243 lb	0.391 (39%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.434 (L/456)	8'9 3/8"	0.823 (L/240)	0.527 (53%)	D	Uniform

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Girders are designed to be supported on the bottom edge only.
3 Top must be laterally braced at end bearings.
4 Bottom must be laterally braced at end bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Point	0-6-8		Top	802 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
2	Point	2-6-8		Top	802 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
3	Point	4-6-8		Top	802 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
4	Point	6-6-8		Top	802 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								

Continued on page 2...

Notes

It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads.

Engineered Wood Products

1. Dry service conditions, unless noted otherwise
2. No treatment with fire-retardant or other strength-reducing chemicals.

Handling & Installation

1. Engineered wood products must not be cut or drilled. Damaged products shall not be used.
2. Refer to the latest version of the installation guide for construction details, hole specifications, multiple-member connections, and handling guidelines.
3. Provide lateral support at bearing points to prevent lateral displacement and rotation.
4. For flat roof, provide proper drainage to prevent ponding.

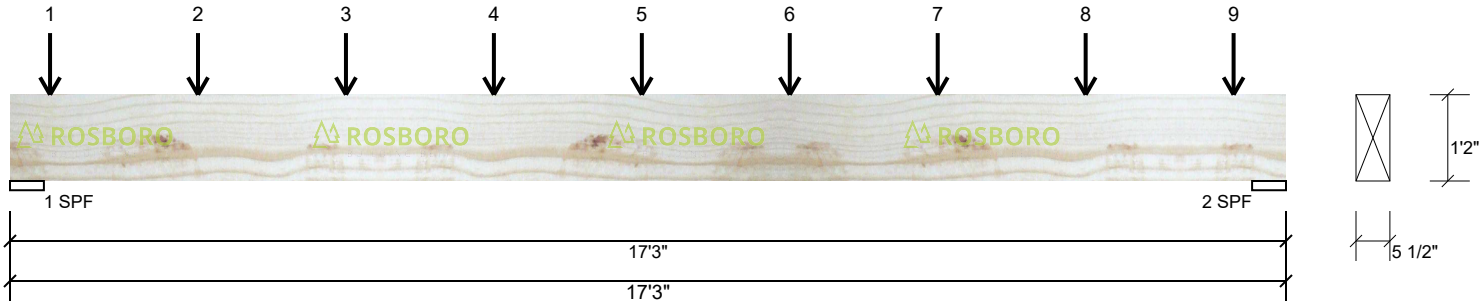
This design is valid until 5/30/2024

Manufacturer Info

Rosboro
Springfield, OR 97477
(877) 457-4139
www.rosboro.com
APA: PR-L275, ICC-ES: ESR-1940

H1 Rosboro 24F-V4 5.500" X 14.000" - PASSED

Level: Level



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ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
5	Point	8-6-8		Top	1386 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
6	Point	10-6-8		Top	1386 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
7	Point	12-6-8		Top	1386 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
8	Point	14-6-8		Top	1386 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
9	Point	16-6-8		Top	1386 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
	Self Weight				19 PLF					

Notes

It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads.

Engineered Wood Products

1. Dry service conditions, unless noted otherwise
2. No treatment with fire-retardant or other strength-reducing chemicals.

Handling & Installation

1. Engineered wood products must not be cut or drilled. Damaged products shall not be used.
2. Refer to the latest version of the installation guide for construction details, hole specifications, multiple-member connections, and handling guidelines.
3. Provide lateral support at bearing points to prevent lateral displacement and rotation.
4. For flat roof, provide proper drainage to prevent ponding.

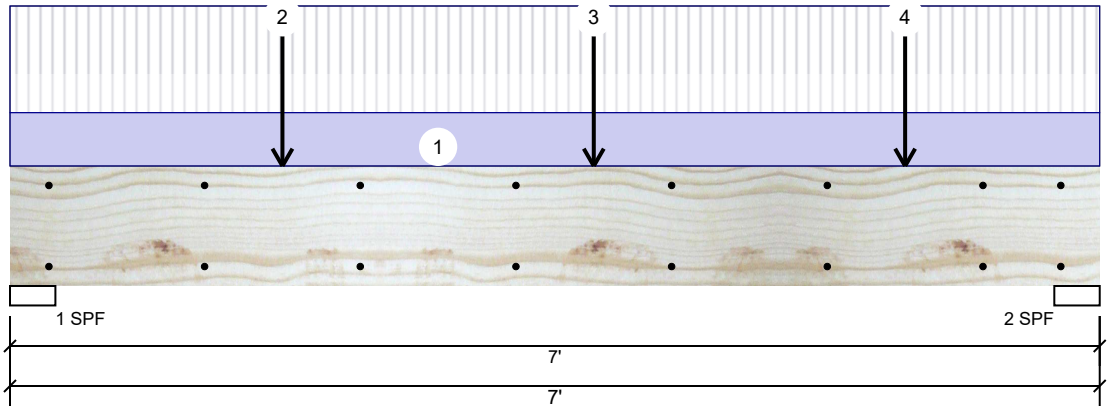
This design is valid until 5/30/2024

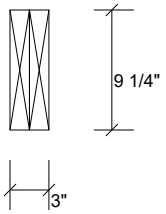
Manufacturer Info

Rosboro
Springfield, OR 97477
(877) 457-4139
www.rosboro.com
APA: PR-L275, ICC-ES: ESR-1940

H2 D FIR-L #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Header	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Header Supports Glass:	No
Importance:	Normal - II	Deck:	Not Checked
Temperature:	Temp <= 100°F		

Reactions PATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	210	931	0	0	0
2	Vertical	210	1067	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	26%	931 / 210	1141	L	D+L
2 - SPF	3.500"	Vert	29%	1067 / 210	1277	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2193 ft-lb	3'9"	3529 ft-lb	0.621 (62%)	D+L	L
Unbraced	2193 ft-lb	3'9"	3280 ft-lb	0.669 (67%)	D+L	L
Shear	1182 lb	5'11 1/4"	3330 lb	0.355 (35%)	D+L	L
LL Defl inch	0.008 (L/10052)	3'6"	0.164 (L/480)	0.048 (5%)	L	L
TL Defl inch	0.051 (L/1527)	3'6 1/8"	0.327 (L/240)	0.157 (16%)	D+L	L

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
3 Refer to last page of calculations for fasteners required for specified loads.
4 Girders are designed to be supported on the bottom edge only.
5 Top loads must be supported equally by all plies.
6 Top must be laterally braced at end bearings.
7 Bottom must be laterally braced at end bearings.
8 Lateral slenderness ratio based on single ply width.

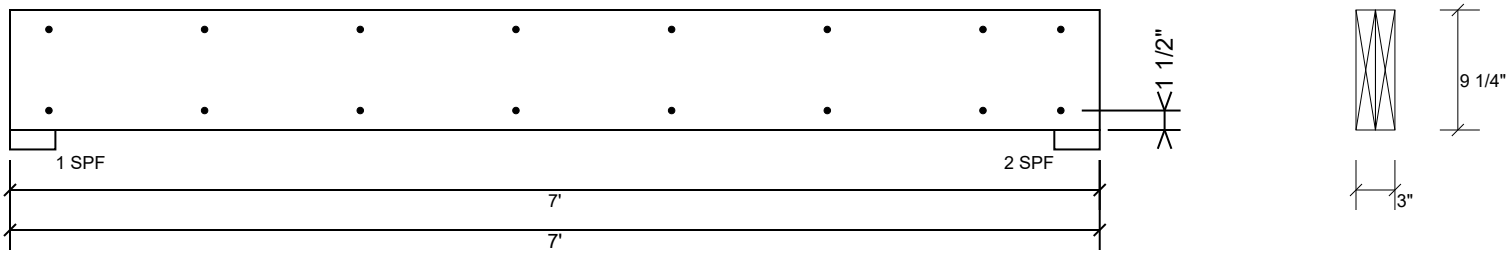
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform		2-0-0	Top	15 PSF	30 PSF	0 PSF	0 PSF	0 PSF	ROOF
2	Point	1-9-0		Top	596 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
3	Point	3-9-0		Top	596 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
4	Point	5-9-0		Top	596 lb	0 lb	0 lb	0 lb	0 lb	TRUSS

Manufacturer Info

This design is valid until 5/30/2024

H2 D FIR-L #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	185.4 PLF
Yield Limit per Fastener	92.7 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

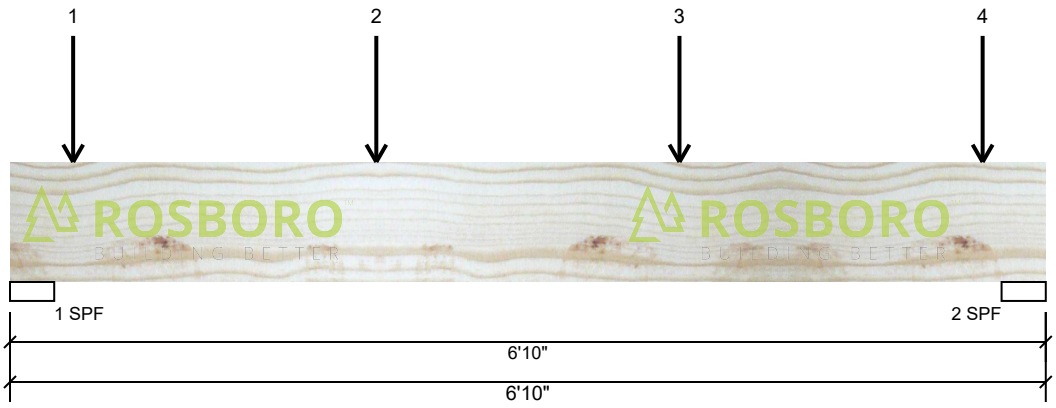
Manufacturer Info

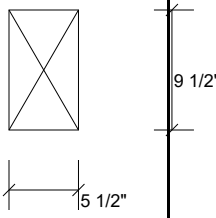
This design is valid until 5/30/2024

Client:		Date:	3/30/2023
Project:		Input by:	
Address:		Job Name:	3150 FC15 BEAMS
		Project #:	

B1 Rosboro 24F-V4 5.500" X 9.500" - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Floor
Moisture Condition:	Dry	Design Method:	ASD
Deflection LL:	480	Building Code:	IBC/IRC 2015
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Wet Use:	No
Temperature:	Temp <= 100°F	Deck:	Not Checked

Reactions PATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	6356	0	0	0
2	Vertical	0	5839	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	78%	6356 / 0	6356	Uniform	D
2 - SPF	3.500"	Vert	71%	5839 / 0	5839	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8647 ft-lb	2'5"	14891 ft-lb	0.581 (58%)	D	Uniform
Unbraced	8647 ft-lb	2'5"	14834 ft-lb	0.583 (58%)	D	Uniform
Shear	4155 lb	1'1"	8308 lb	0.500 (50%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.091 (L/839)	3'5"	0.319 (L/240)	0.286 (29%)	D	Uniform

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Girders are designed to be supported on the bottom edge only.
3 Top must be laterally braced at bearings.
4 Bottom must be laterally braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Point	0-5-0		Top	2610 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
2	Point	2-5-0		Top	3719 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
3	Point	4-5-0		Top	3719 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
4	Point	6-5-0		Top	2060 lb	0 lb	0 lb	0 lb	0 lb	TRUSS
	Bearing Length	0-3-8								
	Self Weight				13 PLF					

Notes

It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application and to verify the dimensions and loads.

Engineered Wood Products

1. Dry service conditions, unless noted otherwise
2. No treatment with fire-retardant or other strength-reducing chemicals.

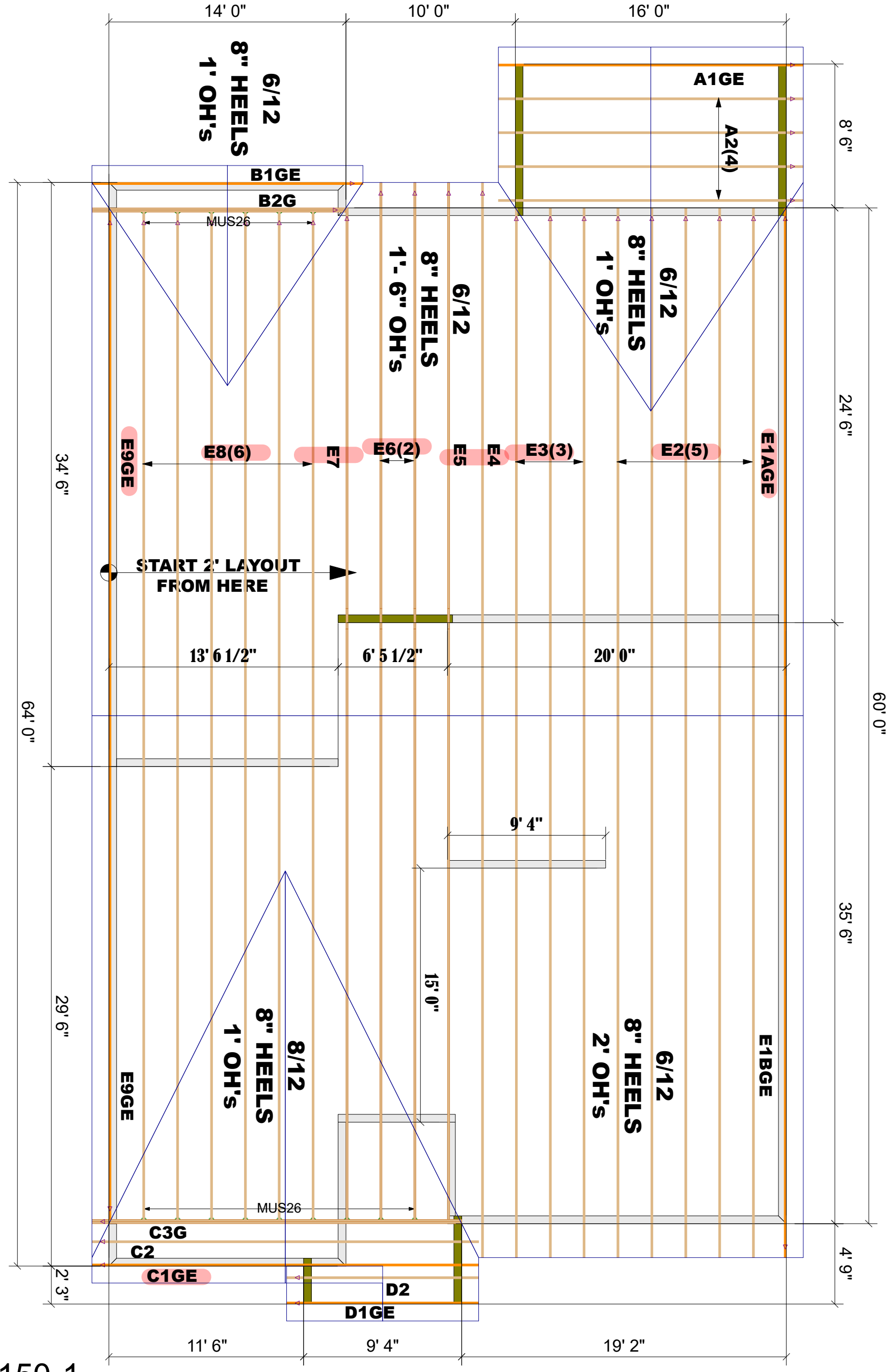
Handling & Installation

1. Engineered wood products must not be cut or drilled. Damaged products shall not be used.
2. Refer to the latest version of the installation guide for construction details, hole specifications, multiple-member connections, and handling guidelines.
3. Provide lateral support at bearing points to prevent lateral displacement and rotation.
4. For flat roof, provide proper drainage to prevent ponding.

This design is valid until 5/30/2024

Manufacturer Info

Rosboro
Springfield, OR 97477
(877) 457-4139
www.rosboro.com
APA: PR-L275, ICC-ES: ESR-1940



J-3150-1

BUILDER: STONE BRIDGE HOMES NW	DATE: 3/28/2023	ABOVE PLAN PROVIDED FOR TRUSS PLACEMENT ONLY. REFER TO TRUSS CALCULATIONS AND ENGINEERED STRUCTURAL DRAWINGS FOR ALL FURTHER INFORMATION. BUILDING DESIGNER/ENGINEER OF RECORD RESPONSIBLE FOR ALL NON-TRUSS TO TRUSS CONNECTION. BUILDING DESIGNER/ENGINEER OF RECORD TO REVIEW AND APPROVE OF ALL DESIGNS PRIOR TO CONSTRUCTION.	Pacific Lumber & TRUSS COMPANY
LOT: 15 SUBDIV: FARMSTEAD CROSSING	LOAD: 25 - 07 - 10		
PLAN: 300-STD	SALES: Jim Andrew		10515 SW ALLEN BLVD. BEAVERTON, OR 97005 PHONE: (503) 858-9663
DEL LOC: xxx TIANA STREET, FOREST GROVE	DES: KH		

MiTek USA, Inc.
MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661
Telephone 916-755-3571

Re: J-3150-1
J-3150

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Pacific Lumber & Truss Co..

Pages or sheets covered by this seal: R75436337 thru R75436355

My license renewal date for the state of Oregon is December 31, 2023.

March 28,2023

Baxter, David

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	A1GE	Common	1	1	R75436337
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:02 Page: 1
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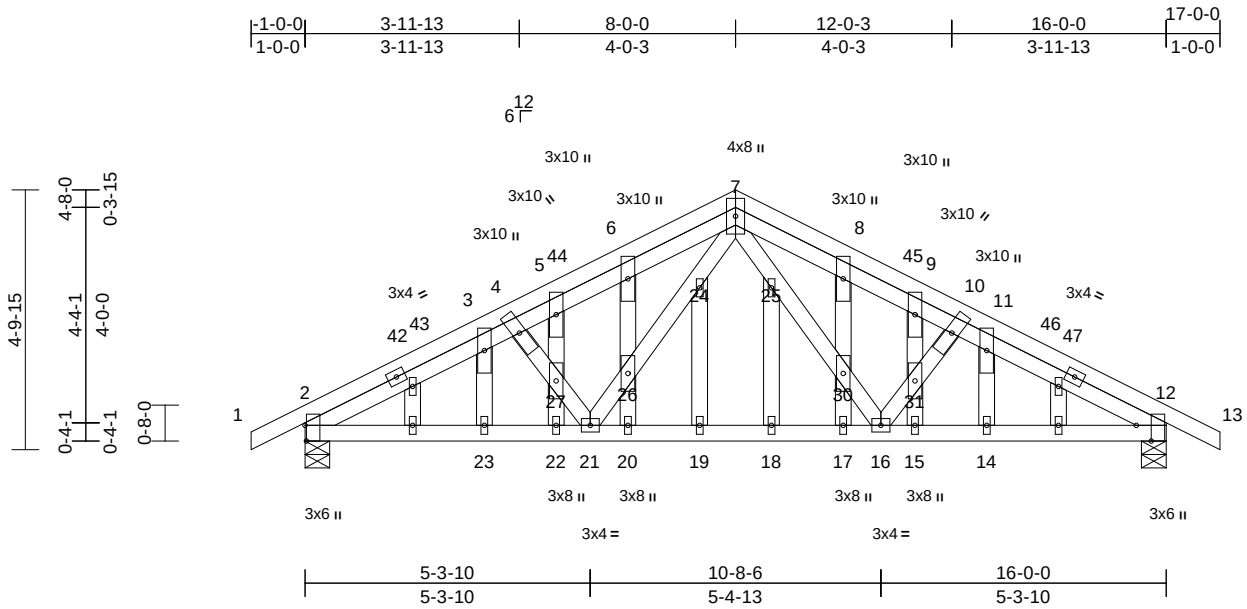


Scale = 1:42.8

Plate Offsets (X, Y): [2:0-3-8,Edge], [12:0-3-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.02	17	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.24	Vert(CT)	-0.04	17	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.02	12	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 118 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 DF Std
OTHERS 2x4 DF Std

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (size) 2=0-5-8, 12=0-5-8
Max Horiz 2=-59 (LC 15)
Max Uplift 2=-71 (LC 14), 12=-71 (LC 15)
Max Grav 2=811 (LC 21), 12=811 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/29, 2-3=-1153/180, 3-4=-1070/198, 4-5=-1010/194, 5-6=-1021/207, 6-7=-1007/239, 7-8=-1007/239, 8-9=-1021/207, 9-10=-1010/194, 10-11=-1070/198, 11-12=-1153/180, 12-13=0/29
BOT CHORD 2-23=-110/983, 22-23=-110/983, 21-22=-110/983, 20-21=-42/655, 19-20=-42/655, 18-19=-42/655, 17-18=-42/655, 16-17=-42/655, 15-16=-113/983, 14-15=-113/983, 12-14=-113/983
WEBS 4-27=-139/55, 21-27=-185/60, 21-26=-76/424, 24-26=-86/424, 7-24=-84/490, 7-25=-84/490, 25-30=-86/424, 16-30=-76/424, 16-31=-185/62, 10-31=-139/57, 19-24=0/77, 18-25=0/77, 6-26=-112/55, 20-26=-112/43, 5-27=-112/23, 22-27=-56/13, 3-23=-9/64, 8-30=-112/55, 17-30=-112/43, 9-31=-112/23, 15-31=-56/13, 11-14=-9/64

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 8-0-0, Exterior(2R) 8-0-0 to 11-0-0, Interior (1) 11-0-0 to 17-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
6) All plates are 1.5x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 1-4-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Solid blocking is required on both sides of the truss at joint(s), 2.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 2 and 71 lb uplift at joint 12.
12) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

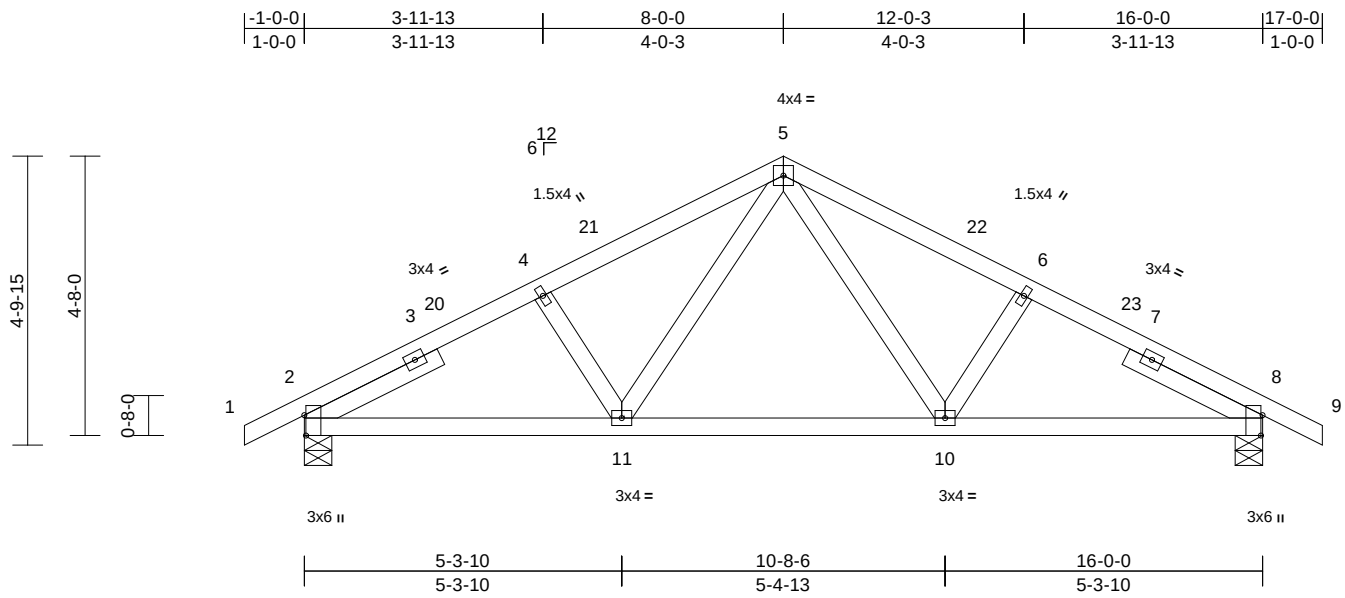
MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	A2	Common	4	1	R75436338

Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:04 ID:eONTm1zQjyyASFmogJ8GDGzWQCw-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:38.5

Plate Offsets (X, Y): [2:0-4-1,Edge], [8:0-4-1,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	-0.03	10-11	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.19	Vert(CT)	-0.05	10-11	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	8	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 76 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std
SLIDER	Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.

REACTIONS (size) 2=0-5-8, 8=0-5-8
Max Horiz 2=-61 (LC 15)
Max Uplift 2=-71 (LC 14), 8=-71 (LC 15)
Max Grav 2=811 (LC 21), 8=811 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/29, 2-4=-1128/199, 4-5=-1025/205, 5-6=-1025/205, 6-8=-1128/199, 8-9=0/29
BOT CHORD	2-11=-117/985, 10-11=-39/621, 8-10=-121/985
WEBS	4-11=-293/116, 5-11=-41/403, 5-10=-41/403, 6-10=-293/116

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 8-0-0, Exterior(2R) 8-0-0 to 11-0-0, Interior (1) 11-0-0 to 17-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 2 and 71 lb uplift at joint 8.
8) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

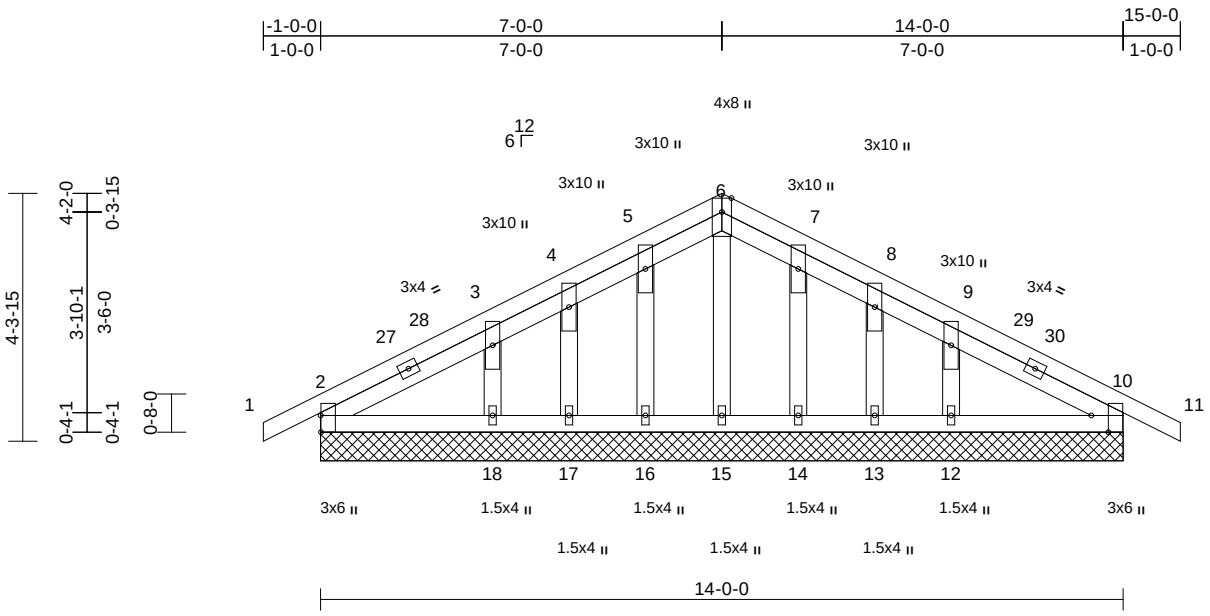
LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	B1GE	Common Supported Gable	1	1	R75436339
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:04 Page: 1
ID:cUmUsr2JCodn5gvslRDbBYzWQAE-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:40.2

Plate Offsets (X, Y): [2:0-3-8,Edge], [10:0-3-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
TCDL	7.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	10	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 85 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
OTHERS 2x4 DF Std

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (size) 2=14-0-0, 10=14-0-0, 12=14-0-0, 13=14-0-0, 14=14-0-0, 15=14-0-0, 16=14-0-0, 17=14-0-0, 18=14-0-0, 19=14-0-0, 23=14-0-0
Max Horiz 2=52 (LC 14), 19=52 (LC 14)
Max Uplift 2=-21 (LC 15), 10=-30 (LC 15), 12=-50 (LC 15), 13=-23 (LC 15), 14=-7 (LC 15), 16=-11 (LC 14), 17=-20 (LC 14), 18=-55 (LC 14), 19=-21 (LC 15), 23=-30 (LC 15)
Max Grav 2=213 (LC 21), 10=213 (LC 22), 12=300 (LC 22), 13=128 (LC 22), 14=158 (LC 22), 15=86 (LC 22), 16=157 (LC 21), 17=128 (LC 21), 18=301 (LC 21), 19=213 (LC 21), 23=213 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/29, 2-3=-73/46, 3-4=-80/85, 4-5=-71/115, 5-6=-68/136, 6-7=-68/136, 7-8=-71/115, 8-9=-80/85, 9-10=-68/33, 10-11=0/29
BOT CHORD 2-18=0/49, 17-18=0/49, 16-17=0/49, 15-16=0/49, 14-15=0/49, 13-14=0/49, 12-13=0/49, 10-12=0/49
WEBS 6-15=-61/10, 5-16=-126/35, 4-17=-116/58, 3-18=-242/114, 7-14=-128/35, 8-13=-116/58, 9-12=-241/113

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 6-11-13, Corner(3R) 6-11-13 to 9-11-13, Exterior(2N) 9-11-13 to 15-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 1-4-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Solid blocking is required on both sides of the truss at joint(s), 2.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 2, 30 lb uplift at joint 10, 11 lb uplift at joint 16, 20 lb uplift at joint 17, 55 lb uplift at joint 18, 7 lb uplift at joint 14, 23 lb uplift at joint 13, 50 lb uplift at joint 12, 21 lb uplift at joint 2 and 30 lb uplift at joint 10.
12) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

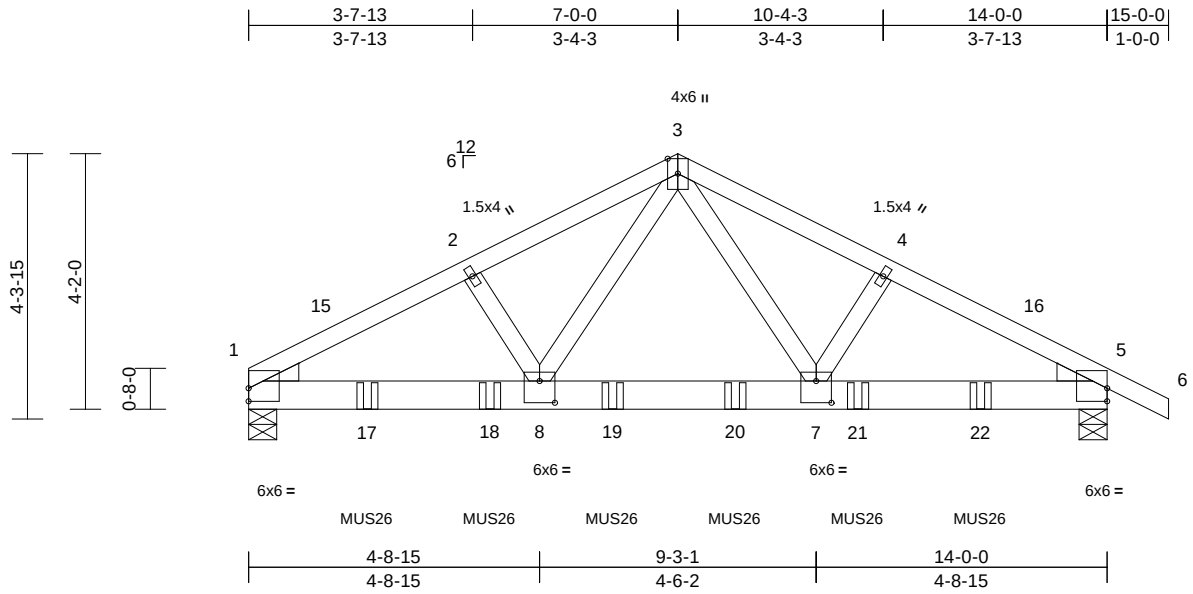
RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	B2G	Common Girder	1	**2**	R75436340
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:05 ID:Y7HSQzdH5VqsAmhexc8D_czWP3m-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:37.6

Plate Offsets (X, Y): [1:Edge,0-2-9], [5:Edge,0-2-9], [7:0-3-0,0-4-4], [8:0-3-0,0-4-4]

Loading	(psf)	**Spacing**	2-0-0	**CSI**		**DEFL**	in	(loc)	l/defl	L/d	**PLATES**	**GRIP**
TCLL	25.0	Plate Grip DOL	1.15	TC	0.26	Vert(LL)	-0.08	7-8	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.12	7-8	>999	180		
TCDL	7.0	Rep Stress Incr	NO	WB	0.65	Horz(CT)	0.02	5	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-MS								
BCDL	10.0										Weight: 142 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x6 DF SS
WEBS 2x4 DF Std
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=0-5-8, 5=0-5-8
Max Horiz 1=-61 (LC 15)
Max Uplift 1=-489 (LC 10), 5=-500 (LC 11)
Max Grav 1=3799 (LC 17), 5=3811 (LC 18)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6000/781, 2-3=-5842/790, 3-4=-5816/785, 4-5=-5973/776, 5-6=0/29
BOT CHORD 1-8=-701/5281, 7-8=-455/3687, 5-7=-642/5254
WEBS 2-8=-195/132, 3-8=-398/2921, 3-7=-391/2881, 4-7=-185/132

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
4) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 489 lb uplift at joint 1 and 500 lb uplift at joint 5.
10) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
11) Use Simpson Strong-Tie MUS26 (6-10d Girder, 4-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-11-4 from the left end to 11-11-4 to connect truss(es) to back face of bottom chord.
12) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-64, 3-6=-64, 9-12=-20
Concentrated Loads (lb)
Vert: 17=-1032 (B), 18=-1032 (B), 19=-1032 (B), 20=-1032 (B), 21=-1032 (B), 22=-1032 (B)

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	C1GE	Common Structural Gable	1	1	R75436341
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:05 Page: 1
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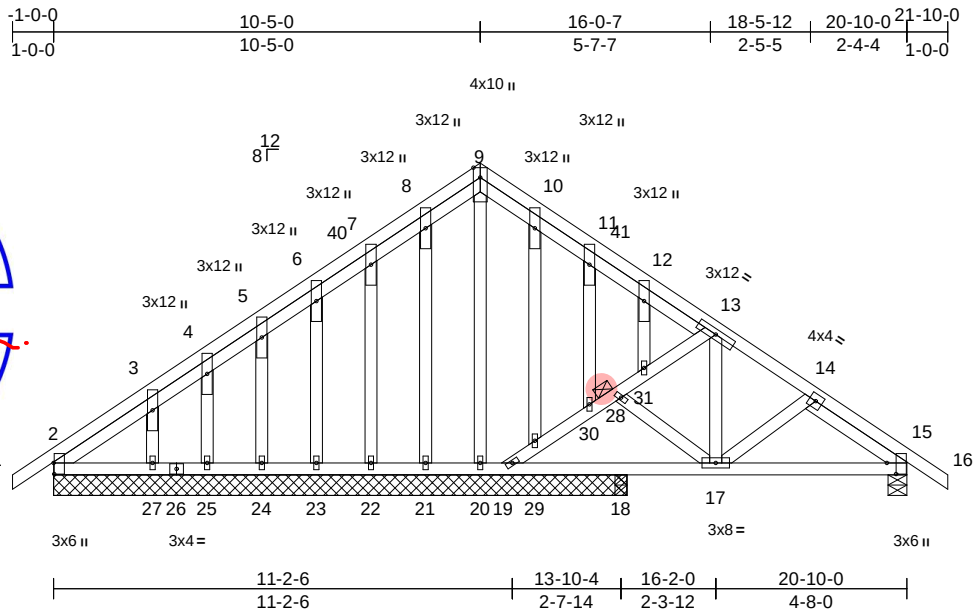

RENEWAL DATE: 12-31-2023

Scale = 1:56.3

Plate Offsets (X, Y): [2:0-3-4,0-0-3], [15:0-3-4,0-2-11]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	-0.01	17-38	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.08	Vert(CT)	-0.01	17-38	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.17	Horz(CT)	0.00	15	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 168 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 DF Std *Except* 13-19:2x4 DF No.1&Btr
OTHERS 2x4 DF Std
SLIDER Right 2x4 DF Std -- 2-9-9

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
JOINTS 1 Brace at Jt(s): 28

REACTIONS (size) 2=14-0-0, 15=0-5-8, 18=0-3-8, 19=14-0-0, 20=14-0-0, 21=14-0-0, 22=14-0-0, 23=14-0-0, 24=14-0-0, 25=14-0-0, 27=14-0-0, 32=14-0-0
Max Horiz 2=-148 (LC 10), 32=-148 (LC 10)
Max Uplift 2=-23 (LC 8), 15=-69 (LC 13), 19=-71 (LC 13), 22=-36 (LC 12), 23=-36 (LC 12), 24=-34 (LC 12), 25=-18 (LC 12), 27=-74 (LC 12), 32=-23 (LC 8)
Max Grav 2=126 (LC 1), 15=426 (LC 1), 18=98 (LC 3), 19=334 (LC 1), 20=214 (LC 1), 21=115 (LC 20), 22=104 (LC 20), 23=110 (LC 20), 24=114 (LC 1), 25=91 (LC 1), 27=197 (LC 20), 32=126 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-120/150, 3-4=-71/114, 4-5=-48/114, 5-6=-30/105, 6-7=-21/109, 7-8=-36/120, 8-9=-41/117, 9-10=-36/123, 10-11=-34/106, 11-12=-22/89, 12-13=-14/78, 13-14=-310/78, 14-15=-277/34, 15-16=0/36
BOT CHORD 2-27=-94/127, 25-27=-94/127, 24-25=-94/127, 23-24=-94/127, 22-23=-94/127, 21-22=-94/127, 20-21=-94/127, 19-20=-95/127, 18-19=-30/329, 17-18=-30/329, 15-17=-19/304
WEBS 13-17=-33/195, 17-28=-127/73, 14-17=-104/56, 19-29=-480/193, 29-30=-452/190, 28-30=-418/164, 28-31=-376/140, 13-31=-333/115, 9-20=-207/0, 8-21=-86/18, 7-22=-77/51, 6-23=-84/53, 5-24=-85/48, 4-25=-74/39, 3-27=-141/81, 10-29=-55/5, 11-30=-62/46, 12-31=-79/45

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 10-4-12, Exterior(2R) 10-4-12 to 13-4-12, Interior (1) 13-4-12 to 21-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Solid blocking is required on both sides of the truss at joint(s), 2.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 2, 71 lb uplift at joint 19, 69 lb uplift at joint 15, 36 lb uplift at joint 22, 36 lb uplift at joint 23, 34 lb uplift at joint 24, 18 lb uplift at joint 25, 74 lb uplift at joint 27 and 23 lb uplift at joint 2.
11) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	C2	Common	1	1	R75436342
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:06 Page: 1
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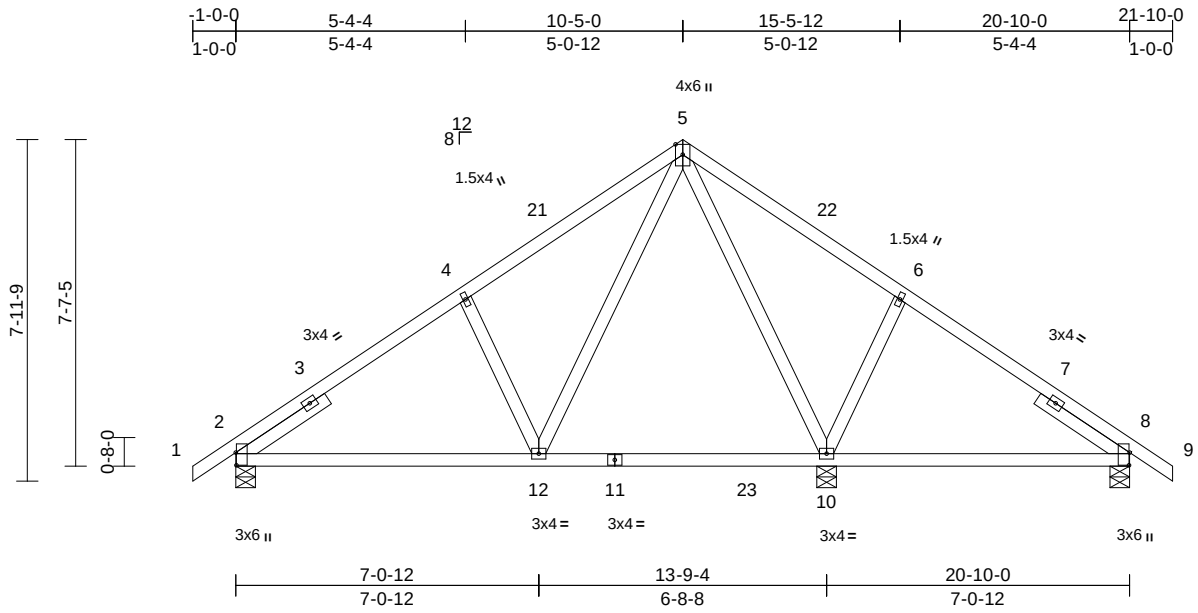


Scale = 1:53.7

Plate Offsets (X, Y): [2:0-3-9,0-0-3], [8:0-3-9,0-0-3]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	-0.05	10-12	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.27	Vert(CT)	-0.07	10-12	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.46	Horz(CT)	0.01	10	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 104 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 DF Std
SLIDER Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (size) 2=0-5-8, 8=0-5-8, 10=0-5-8
Max Horiz 2=152 (LC 11)
Max Uplift 2=-69 (LC 12), 8=-64 (LC 13), 10=-30 (LC 12)
Max Grav 2=725 (LC 20), 8=458 (LC 21), 10=843 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-4=-704/82, 4-5=-690/132, 5-6=-255/124, 6-8=-418/73, 8-9=0/36
BOT CHORD 2-12=-137/697, 10-12=-2/323, 8-10=-49/239
WEBS 4-12=-299/170, 5-12=-86/595, 5-10=-411/53, 6-10=-322/171

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 10-5-0, Exterior(2R) 10-5-0 to 13-5-0, Interior (1) 13-5-0 to 21-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 2, 30 lb uplift at joint 10 and 64 lb uplift at joint 8.
7) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

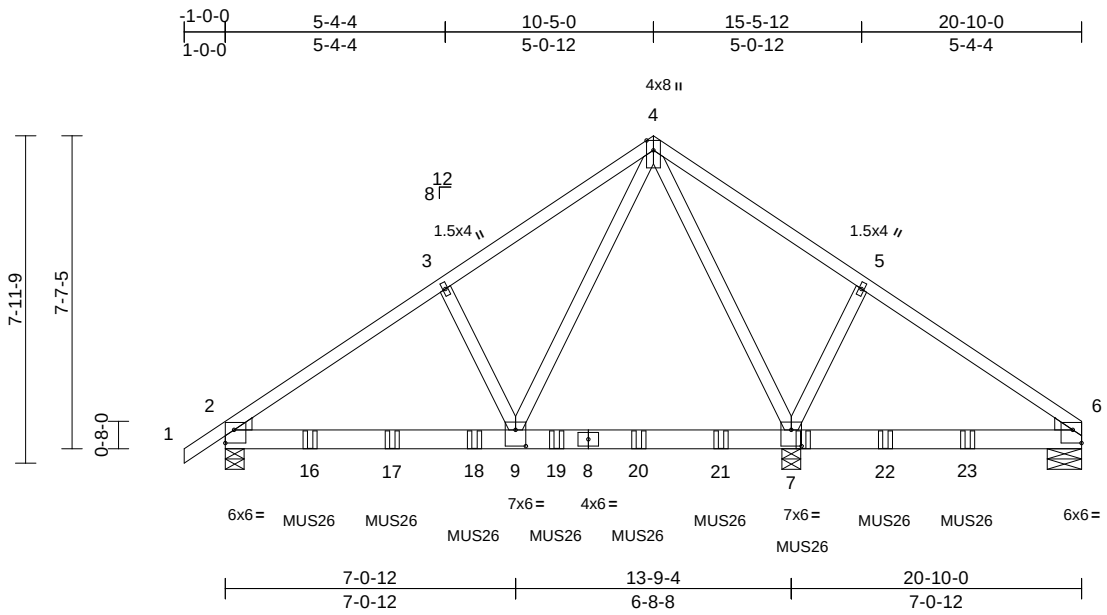
MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	C3G	Common Girder	1	**2**	R75436343

Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:06 ID:7fvKpF9lq0TAX0MCo_yc1tzWP5g-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:56.1

Plate Offsets (X, Y): [2:Edge,0-3-14], [6:Edge,0-3-14], [7:0-3-0,0-4-12], [9:0-3-0,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.08	9-12	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.12	9-12	>999	180		
TCDL	7.0	Rep Stress Incr	NO	WB	0.74	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-MS								
BCDL	10.0										Weight: 225 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x6 DF SS
WEBS 2x4 DF Std
WEDGE Left: 2x4 DF Std
Right: 2x4 DF Std

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-5-8, 6=0-10-0, 7=0-5-8
Max Horiz 2=148 (LC 7)
Max Uplift 2=-455 (LC 8), 6=-40 (LC 9), 7=-598 (LC 9)
Max Grav 2=2636 (LC 1), 6=347 (LC 1), 7=3452 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-2950/517, 3-4=-2853/568, 4-5=-174/95, 5-6=-294/45
BOT CHORD 2-9=-463/2433, 7-9=-147/882, 6-7=0/188
WEBS 3-9=-346/180, 4-9=-656/3345, 4-7=-1933/358, 5-7=-304/172

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
4) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 455 lb uplift at joint 2, 598 lb uplift at joint 7 and 40 lb uplift at joint 6.
9) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
10) Use Simpson Strong-Tie MUS26 (6-10d Girder, 4-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 18-0-12 to connect truss(es) to back face of bottom chord.
11) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-64, 4-6=-64, 10-13=-20
Concentrated Loads (lb)
Vert: 7=-155 (B), 16=-696 (B), 17=-696 (B), 18=-696 (B), 19=-696 (B), 20=-696 (B), 21=-696 (B), 22=-145 (B), 23=-145 (B)

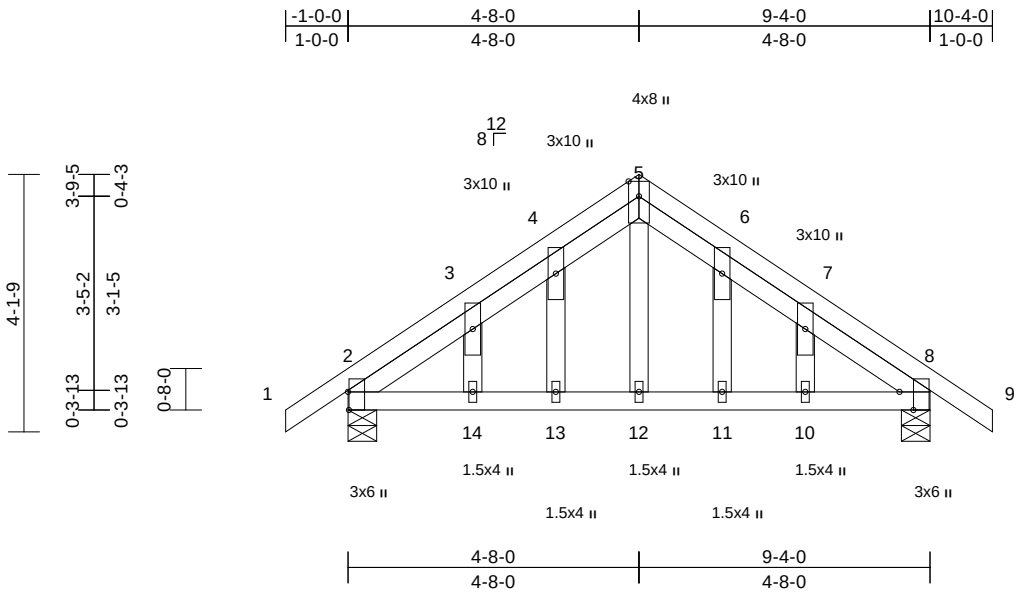
REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	D1GE	Common Structural Gable	1	1	R75436344
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:07 Page: 1
ID:0t1vCJiBUOQ6a_2EvmBGNwzWQ9O-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:37

Plate Offsets (X, Y): [2:0-3-8,Edge], [8:0-3-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	0.01	13-14	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.08	Vert(CT)	-0.01	10-11	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	8	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 59 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 DF Std
OTHERS 2x4 DF Std

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (size) 2=0-5-8, 8=0-5-8
Max Horiz 2=72 (LC 11)
Max Uplift 2=-46 (LC 12), 8=-46 (LC 13)
Max Grav 2=456 (LC 1), 8=456 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-422/76, 3-4=-381/98, 4-5=-334/119, 5-6=-334/119, 6-7=-381/98, 7-8=-422/76, 8-9=0/36
BOT CHORD 2-14=-23/315, 13-14=-4/315, 12-13=-4/315, 11-12=-4/315, 10-11=-4/315, 8-10=-4/315
WEBS 5-12=-14/95, 4-13=0/40, 3-14=-16/32, 6-11=0/40, 7-10=-16/32

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 4-8-0, Exterior(2R) 4-8-0 to 7-8-0, Interior (1) 7-8-0 to 10-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) Solid blocking is required on both sides of the truss at joint(s), 2.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 2 and 46 lb uplift at joint 8.
10) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

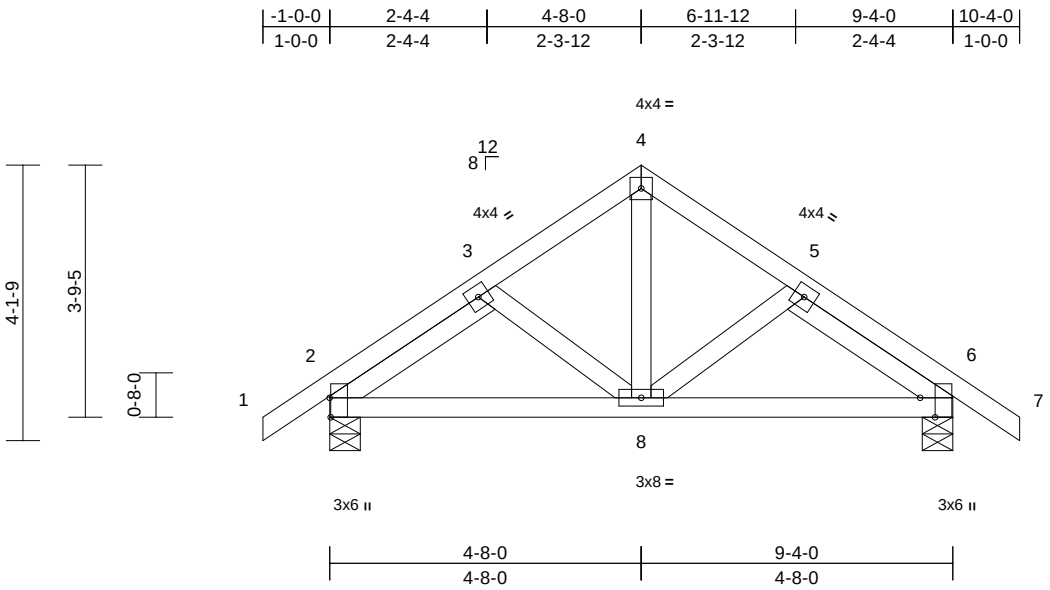
LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	D2	Common	1	1	R75436345
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:07 Page: 1
ID:koehIkqT8ShhmWp9UtNcn1zWQ9E-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:34.5

Plate Offsets (X, Y): [2:0-3-8,Edge], [6:0-3-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	-0.01	8-15	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.01	8-15	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 51 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 DF Std
SLIDER Left 2x4 DF Std -- 2-9-9, Right 2x4 DF Std -- 2-9-9

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (size) 2=0-5-8, 6=0-5-8
Max Horiz 2=76 (LC 11)
Max Uplift 2=-45 (LC 12), 6=-45 (LC 13)
Max Grav 2=456 (LC 1), 6=456 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-277/54, 3-4=-365/107, 4-5=-365/107, 5-6=-277/55, 6-7=0/36
BOT CHORD 2-8=-46/345, 6-8=-34/344
WEBS 4-8=-43/220, 3-8=-105/77, 5-8=-106/77

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 4-8-0, Exterior(2R) 4-8-0 to 7-8-0, Interior (1) 7-8-0 to 10-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 2 and 45 lb uplift at joint 6.
7) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

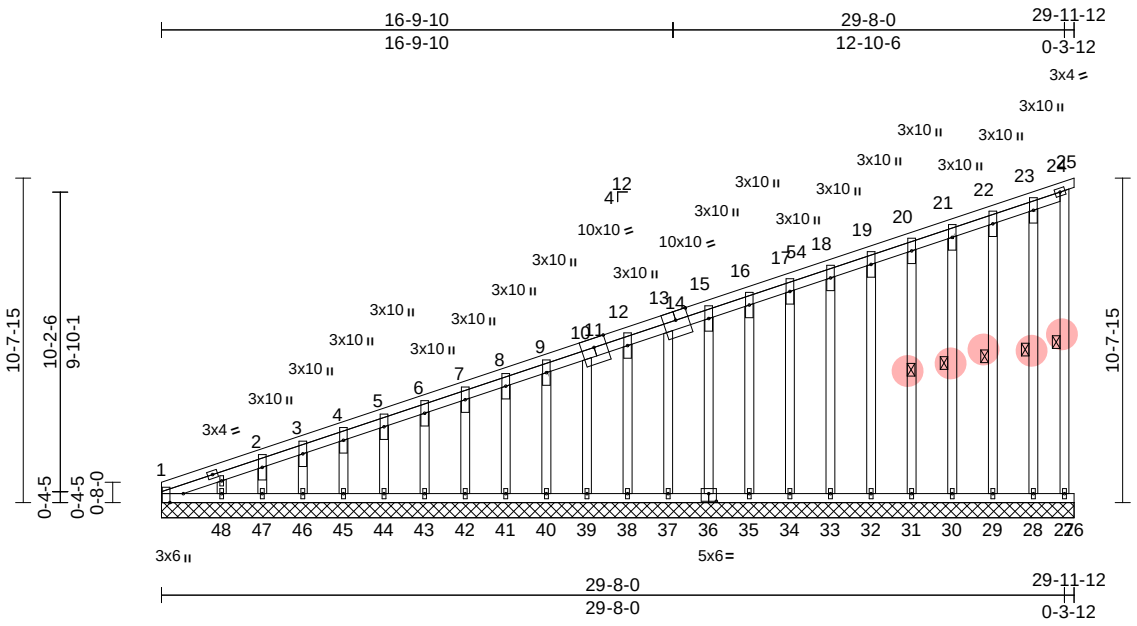
LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E1AGE	Monopitch Supported Gable	1	1	R75436346
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:07 Page: 1
ID:SoDyE17R7HKPQgbHfxr3TYzWP1r-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:75.7

Plate Offsets (X, Y): [1:0-3-8,Edge], [36:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.04	Vert(TL)	n/a	-	n/a	999		
TCDL	7.0	Rep Stress Incr	YES	WB	0.15	Horiz(TL)	-0.01	25	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 279 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 DF Std
OTHERS 2x4 DF Std

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 24-27, 23-28, 22-29, 21-30, 20-31

REACTIONS (size) 1=29-11-12, 25=29-11-12, 26=29-11-12, 27=29-11-12, 28=29-11-12, 29=29-11-12, 30=29-11-12, 31=29-11-12, 32=29-11-12, 33=29-11-12, 34=29-11-12, 35=29-11-12, 36=29-11-12, 37=29-11-12, 38=29-11-12, 39=29-11-12, 40=29-11-12, 41=29-11-12, 42=29-11-12, 43=29-11-12, 44=29-11-12, 45=29-11-12, 46=29-11-12, 47=29-11-12, 48=29-11-12, 50=29-11-12
Max Horiz 1=312 (LC 10), 50=312 (LC 10)
Max Uplift 25=-14 (LC 10), 26=-1 (LC 5), 28=-14 (LC 10), 29=-20 (LC 10), 30=-20 (LC 10), 31=-19 (LC 10), 32=-19 (LC 10), 33=-19 (LC 10), 34=-19 (LC 10), 35=-19 (LC 10), 36=-19 (LC 10), 37=-19 (LC 10), 38=-19 (LC 10), 39=-19 (LC 10), 40=-19 (LC 10), 41=-19 (LC 10), 42=-19 (LC 10), 43=-19 (LC 10), 44=-21 (LC 10), 45=-17 (LC 10), 47=-54 (LC 10), 48=-56 (LC 10)
Max Grav 1=139 (LC 23), 25=50 (LC 20), 26=0 (LC 10), 27=49 (LC 20), 28=136 (LC 20), 29=173 (LC 20), 30=171 (LC 20), 31=168 (LC 20), 32=166 (LC 20), 33=156 (LC 20), 34=128 (LC 20), 35=112 (LC 1), 36=112 (LC 1), 37=112 (LC 1), 38=112 (LC 20), 39=112 (LC 20), 40=112 (LC 1), 41=112 (LC 1), 42=112 (LC 1), 43=112 (LC 20), 44=113 (LC 20), 45=110 (LC 1), 46=87 (LC 1), 47=189 (LC 1), 48=79 (LC 1), 50=139 (LC 23)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-316/168, 2-3=-283/146, 3-4=-278/146, 4-5=-263/138, 5-6=-248/131, 6-7=-234/123, 7-8=-219/115, 8-9=-205/108, 9-10=-190/100, 10-12=-175/92, 12-13=-161/85, 13-15=-146/77, 15-16=-131/69, 16-17=-117/62, 17-18=-102/54, 18-19=-87/46, 19-20=-73/39, 20-21=-58/31, 21-22=-43/23, 22-23=-28/15, 23-24=-19/18, 24-25=-5/15
BOT CHORD 1-48=0/0, 47-48=0/0, 46-47=0/0, 45-46=0/0, 44-45=0/0, 43-44=0/0, 42-43=0/0, 41-42=0/0, 40-41=0/0, 39-40=0/0, 38-39=0/0, 37-38=0/0, 35-37=0/0, 34-35=0/0, 33-34=0/0, 32-33=0/0, 31-32=0/0, 30-31=0/0, 29-30=0/0, 28-29=0/0, 27-28=0/0, 26-27=0/0
WEBS 24-27=-34/9, 23-28=-112/28, 22-29=-146/37, 21-30=-144/36, 20-31=-141/35, 19-32=-140/35, 18-33=-129/35, 17-34=-101/35, 16-35=-85/35, 15-36=-85/35, 13-37=-85/35, 12-38=-85/35, 10-39=-85/35, 9-40=-85/35, 8-41=-85/35, 7-42=-85/35, 6-43=-85/35, 5-44=-86/36, 4-45=-84/35, 3-46=-55/5, 2-47=-181/96

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-0-0 to 3-0-0, Exterior(2N) 3-0-0 to 29-11-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) Unbalanced snow loads have been considered for this design.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 1-4-0 oc.

RENEWAL DATE: 12-31-2023
March 28,2023

Continued on page 2

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

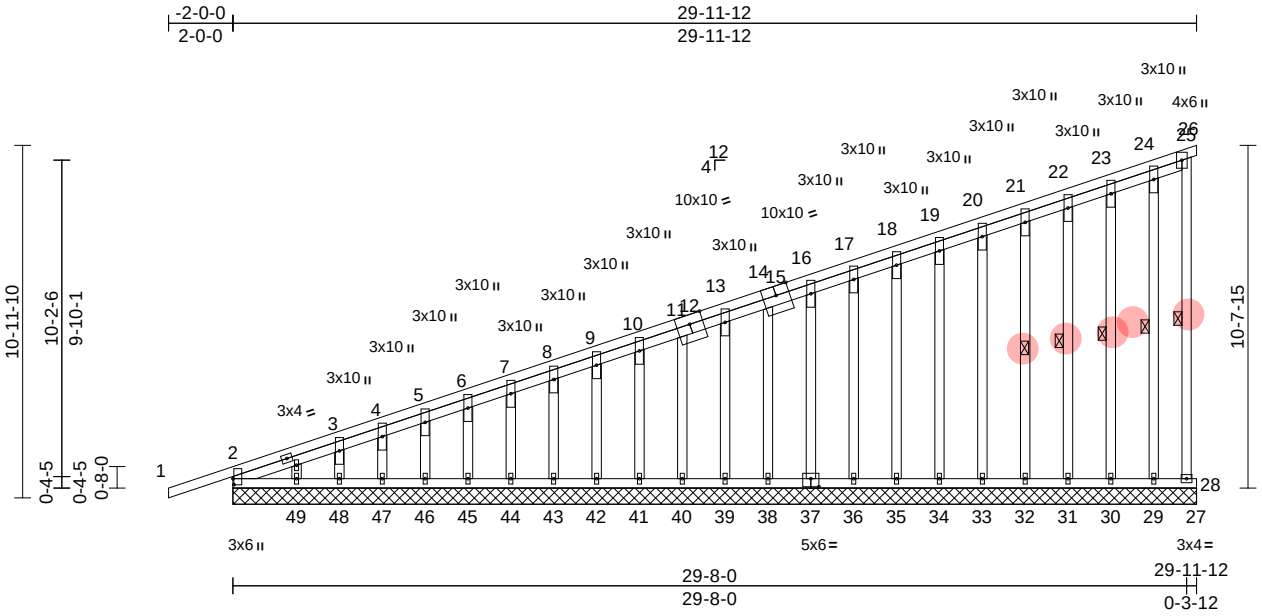
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Bearing at joint(s) 25 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 26, 14 lb uplift at joint 25, 14 lb uplift at joint 28, 20 lb uplift at joint 29, 20 lb uplift at joint 30, 19 lb uplift at joint 31, 19 lb uplift at joint 32, 19 lb uplift at joint 33, 19 lb uplift at joint 34, 19 lb uplift at joint 35, 19 lb uplift at joint 36, 19 lb uplift at joint 37, 19 lb uplift at joint 38, 19 lb uplift at joint 39, 19 lb uplift at joint 40, 19 lb uplift at joint 41, 19 lb uplift at joint 42, 19 lb uplift at joint 43, 21 lb uplift at joint 44, 17 lb uplift at joint 45, 54 lb uplift at joint 47 and 56 lb uplift at joint 48.
12) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E1BGE	Monopitch Supported Gable	1	1	R75436347
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:08 Page: 1

ID:_0UQCVUJoO8Y3lzQ5UlxwzzWOww-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:71.7

Plate Offsets (X, Y): [2:0-2-4,0-0-5], [37:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	n/a	-	n/a	999	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.13	Vert(CT)	n/a	-	n/a	999		
TCDL	7.0	Rep Stress Incr	YES	WB	0.15	Horz(CT)	-0.01	26	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 282 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std
OTHERS	2x4 DF Std

BRACING

TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 25-28, 24-29, 23-30, 22-31, 21-32

REACTIONS (size) 2=29-11-12, 26=29-11-12, 27=29-11-12, 28=29-11-12, 29=29-11-12, 30=29-11-12, 31=29-11-12, 32=29-11-12, 33=29-11-12, 34=29-11-12, 35=29-11-12, 36=29-11-12, 37=29-11-12, 38=29-11-12, 39=29-11-12, 40=29-11-12, 41=29-11-12, 42=29-11-12, 43=29-11-12, 44=29-11-12, 45=29-11-12, 46=29-11-12, 47=29-11-12, 48=29-11-12, 49=29-11-12, 51=29-11-12

Max Horiz 2=358 (LC 13), 51=358 (LC 13)

Max Uplift 2=-10 (LC 10), 26=-84 (LC 10), 27=-370 (LC 13), 28=-215 (LC 10), 29=-11 (LC 11), 30=-26 (LC 11), 31=-24 (LC 10), 32=-21 (LC 10), 33=-20 (LC 10), 34=-19 (LC 14), 35=-19 (LC 14), 36=-19 (LC 14), 37=-19 (LC 10), 38=-19 (LC 10), 39=-19 (LC 14), 40=-19 (LC 10), 41=-19 (LC 14), 42=-19 (LC 10), 43=-19 (LC 14), 44=-19 (LC 10), 45=-20 (LC 14), 46=-18 (LC 10), 47=-6 (LC 10), 48=-47 (LC 14), 49=-34 (LC 11), 51=-10 (LC 10)

Max Grav 2=292 (LC 21), 26=91 (LC 13), 27=278 (LC 10), 28=302 (LC 13), 29=137 (LC 21), 30=175 (LC 21), 31=173 (LC 21), 32=169 (LC 21), 33=169 (LC 21), 34=164 (LC 21), 35=140 (LC 21), 36=115 (LC 21), 37=112 (LC 1), 38=112 (LC 1), 39=112 (LC 21), 40=112 (LC 21), 41=112 (LC 21), 42=112 (LC 1), 43=112 (LC 1), 44=112 (LC 21), 45=112 (LC 1), 46=112 (LC 21), 47=99 (LC 21), 48=171 (LC 1), 49=63 (LC 5), 51=292 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-373/220, 3-4=-343/202, 4-5=-338/203, 5-6=-324/198, 6-7=-310/192, 7-8=-296/186, 8-9=-282/181, 9-10=-269/175, 10-11=-255/169, 11-13=-241/164, 13-14=-227/158, 14-16=-213/153, 16-17=-199/147, 17-18=-186/141, 18-19=-172/136, 19-20=-158/130, 20-21=-149/125, 21-22=-143/120, 22-23=-138/114, 23-24=-125/113, 24-25=-83/83, 25-26=-40/35, 25-28=-37/35

BOT CHORD 2-49=-118/136, 48-49=-118/136, 47-48=-118/136, 46-47=-118/136, 45-46=-118/136, 44-45=-118/136, 43-44=-118/136, 42-43=-118/136, 41-42=-118/136, 40-41=-118/136, 39-40=-118/136, 38-39=-118/136, 36-38=-118/136, 35-36=-118/136, 34-35=-118/136, 33-34=-118/136, 32-33=-118/136, 31-32=-118/136, 30-31=-118/136, 29-30=-118/136, 28-29=-118/136, 27-28=0/0

WEBS 24-29=-113/74, 23-30=-148/49, 22-31=-146/38, 21-32=-142/38, 20-33=-142/36, 19-34=-138/35, 18-35=-114/35, 17-36=-88/35, 16-37=-85/35, 14-38=-85/35, 13-39=-85/35, 11-40=-85/35, 10-41=-85/35, 9-42=-85/35, 8-43=-85/35, 7-44=-85/35, 6-45=-86/36, 5-46=-84/35, 4-47=-74/22, 3-48=-139/85

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 0-8-10, Exterior(2N) 0-8-10 to 29-11-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

RENEWAL DATE: 12-31-2023

March 28,2023

Continued on page 2

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

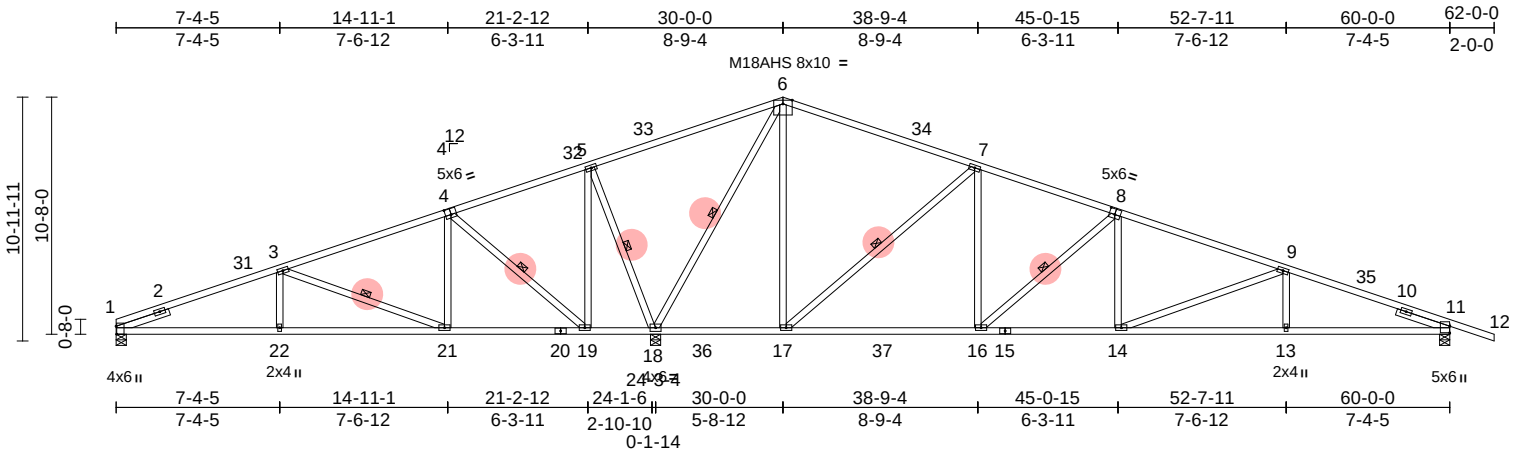
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 20.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
6) All plates are 1.5x4 MT20 unless otherwise indicated.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 1-4-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
11) Bearing at joint(s) 26 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 27, 84 lb uplift at joint 26, 215 lb uplift at joint 28, 11 lb uplift at joint 29, 26 lb uplift at joint 30, 24 lb uplift at joint 31, 21 lb uplift at joint 32, 20 lb uplift at joint 33, 19 lb uplift at joint 34, 19 lb uplift at joint 35, 19 lb uplift at joint 36, 19 lb uplift at joint 37, 19 lb uplift at joint 38, 19 lb uplift at joint 39, 19 lb uplift at joint 40, 19 lb uplift at joint 41, 19 lb uplift at joint 42, 19 lb uplift at joint 43, 19 lb uplift at joint 44, 20 lb uplift at joint 45, 18 lb uplift at joint 46, 6 lb uplift at joint 47, 47 lb uplift at joint 48, 34 lb uplift at joint 49, 10 lb uplift at joint 2 and 10 lb uplift at joint 2.
13) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
14) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E2	Common	5	1	R75436348
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:09 ID:?N7uP4YvGH2QK0QR_CcrrmzWPlm-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:103.7

Plate Offsets (X, Y): [1:0-4-5,Edge], [4:0-3-0,0-3-4], [8:0-3-0,0-3-0], [11:0-3-9,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.91	Vert(LL)	-0.25	13-14	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.40	13-14	>999	180	M18AHS	169/162
TCDL	7.0	Rep Stress Incr	YES	WB	1.00	Horz(CT)	0.06	11	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 314 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std *Except* 18-6:2x4 DF 2400F 2.0E
SLIDER	Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 8-16, 3-21, 5-18, 7-17, 4-19, 6-18

REACTIONS (size) 1=0-5-8, 11=0-5-8, 18=0-5-8
Max Horiz 1=-153 (LC 15)
Max Uplift 1=-91 (LC 14), 11=-227 (LC 11), 18=-265 (LC 10)
Max Grav 1=596 (LC 21), 11=1386 (LC 22), 18=3655 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-3=-946/185, 3-5=-47/1371, 5-6=-51/1835, 6-7=-22/437, 7-9=-1968/301, 9-11=-2734/362, 11-12=0/40
BOT CHORD	1-22=-254/897, 21-22=-254/897, 19-21=-724/253, 18-19=-1259/255, 17-18=-330/193, 16-17=-20/1115, 14-16=-142/1800, 13-14=-274/2548, 11-13=-274/2548
WEBS	6-17=-62/1409, 8-14=0/516, 9-14=-806/142, 9-13=0/278, 7-16=-12/955, 8-16=-963/160, 3-22=0/307, 4-21=0/625, 3-21=-1059/182, 5-19=-46/814, 5-18=-1384/246, 7-17=-1663/248, 4-19=-1043/159, 6-18=-2808/281

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 6-0-0, Interior (1) 6-0-0 to 30-0-0, Exterior(2R) 30-0-0 to 36-0-0, Interior (1) 36-0-0 to 62-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 20.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
6) All plates are MT20 plates unless otherwise indicated.
7) All plates are 3x6 MT20 unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 11, 265 lb uplift at joint 18 and 91 lb uplift at joint 1.
11) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

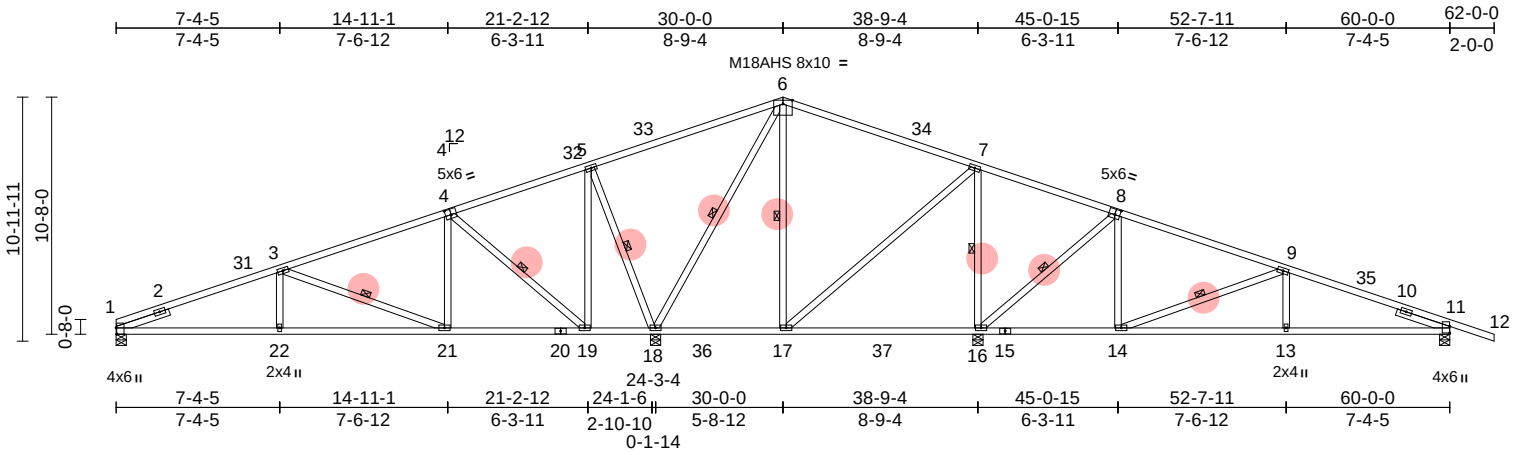
RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E3	Common	3	1	R75436349
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:09 ID:tcJY6qB5K3GgkP7gDv4pdRzWPmE-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:103.7

Plate Offsets (X, Y): [1:0-3-8,Edge], [4:0-3-0,0-3-0], [8:0-2-12,0-3-0], [11:0-3-9,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.14	16-17	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.23	16-17	>768	180	M18AHS	169/162
TCDL	7.0	Rep Stress Incr	YES	WB	0.71	Horz(CT)	0.03	18	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 314 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std
SLIDER	Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 6-17, 7-16, 8-16, 9-14, 3-21, 4-19, 5-18, 6-18

REACTIONS (size) 1=0-5-8, 11=0-5-8, 16=0-5-8, 18=0-5-8
Max Horiz 1=-153 (LC 15)
Max Uplift 1=-83 (LC 10), 11=-168 (LC 11), 16=-197 (LC 15), 18=-276 (LC 10)
Max Grav 1=767 (LC 3), 11=802 (LC 22), 16=2001 (LC 4), 18=2349 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-3=-1375/194, 3-5=-498/541, 5-6=-26/1010, 6-7=0/480, 7-9=-242/794, 9-11=-1094/201, 11-12=0/40
BOT CHORD	1-22=-224/1305, 21-22=-224/1305, 19-21=-54/405, 18-19=-449/171, 17-18=-372/197, 16-17=-701/174, 14-16=-41/143, 13-14=-124/1038, 11-13=-124/1038
WEBS	6-17=-156/188, 7-16=-1174/183, 7-17=0/515, 8-14=0/563, 8-16=-989/163, 9-13=0/307, 9-14=-984/162, 3-22=0/293, 3-21=-972/181, 4-21=0/602, 4-19=-1028/159, 5-19=-46/808, 5-18=-1378/246, 6-18=-1110/112

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 6-0-0, Interior (1) 6-0-0 to 30-0-0, Exterior(2R) 30-0-0 to 36-0-0, Interior (1) 36-0-0 to 62-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 20.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
6) All plates are MT20 plates unless otherwise indicated.
7) All plates are 3x6 MT20 unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 1, 197 lb uplift at joint 16, 168 lb uplift at joint 11 and 276 lb uplift at joint 18.
11) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E4	Common	1	1	R75436350

Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:10 Page: 1

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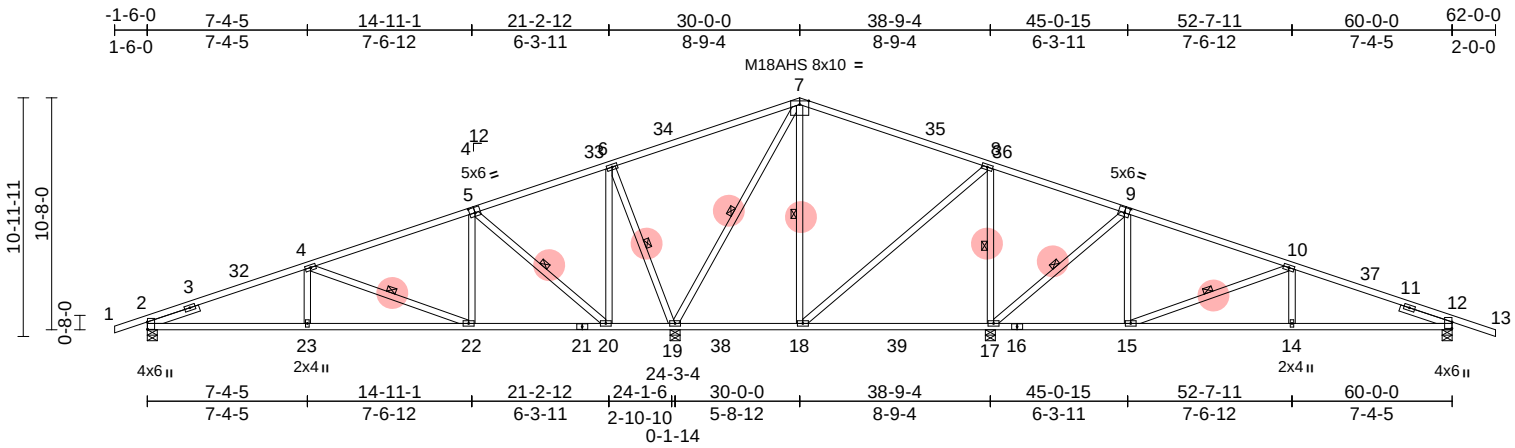

Scale = 1:106

Plate Offsets (X, Y): [2:0-4-1,0-0-1], [5:0-3-0,0-3-0], [9:0-2-12,0-3-0], [12:0-3-9,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.87	Vert(LL)	-0.14	17-18	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.23	17-18	>768	180	M18AHS	169/162
TCDL	7.0	Rep Stress Incr	YES	WB	0.72	Horz(CT)	0.03	19	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 316 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std
SLIDER	Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 7-18, 10-15, 8-17, 9-17, 4-22, 6-19, 5-20, 7-19

REACTIONS (size) 2=0-5-8, 12=0-5-8, 17=0-5-8, 19=0-5-8
Max Horiz 2=-146 (LC 19)
Max Uplift 2=-132 (LC 10), 12=-168 (LC 11), 17=-197 (LC 15), 19=-280 (LC 10)
Max Grav 2=846 (LC 21), 12=802 (LC 22), 17=2013 (LC 4), 19=2354 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/30, 2-4=-1346/163, 4-6=-480/550, 6-7=-29/1023, 7-8=0/487, 8-10=-242/797, 10-12=-1094/201, 12-13=0/40
BOT CHORD	2-23=-214/1277, 22-23=-214/1277, 20-22=-50/395, 19-20=-460/173, 18-19=-379/198, 17-18=-704/175, 15-17=-45/143, 14-15=-123/1038, 12-14=-123/1038
WEBS	7-18=-155/189, 9-15=0/563, 10-14=0/307, 10-15=-984/162, 8-17=-1191/182, 9-17=-989/163, 4-23=0/292, 5-22=0/598, 4-22=-956/176, 6-20=-45/806, 6-19=-1374/245, 8-18=0/513, 5-20=-1025/157, 7-19=-1119/119

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-0 to 4-6-0, Interior (1) 4-6-0 to 30-0-0, Exterior(2R) 30-0-0 to 36-0-0, Interior (1) 36-0-0 to 62-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 20.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 3x6 MT20 unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 12, 132 lb uplift at joint 2, 197 lb uplift at joint 17 and 280 lb uplift at joint 19.
10) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E5	Roof Special	1	1	R75436351
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:10 Page: 1
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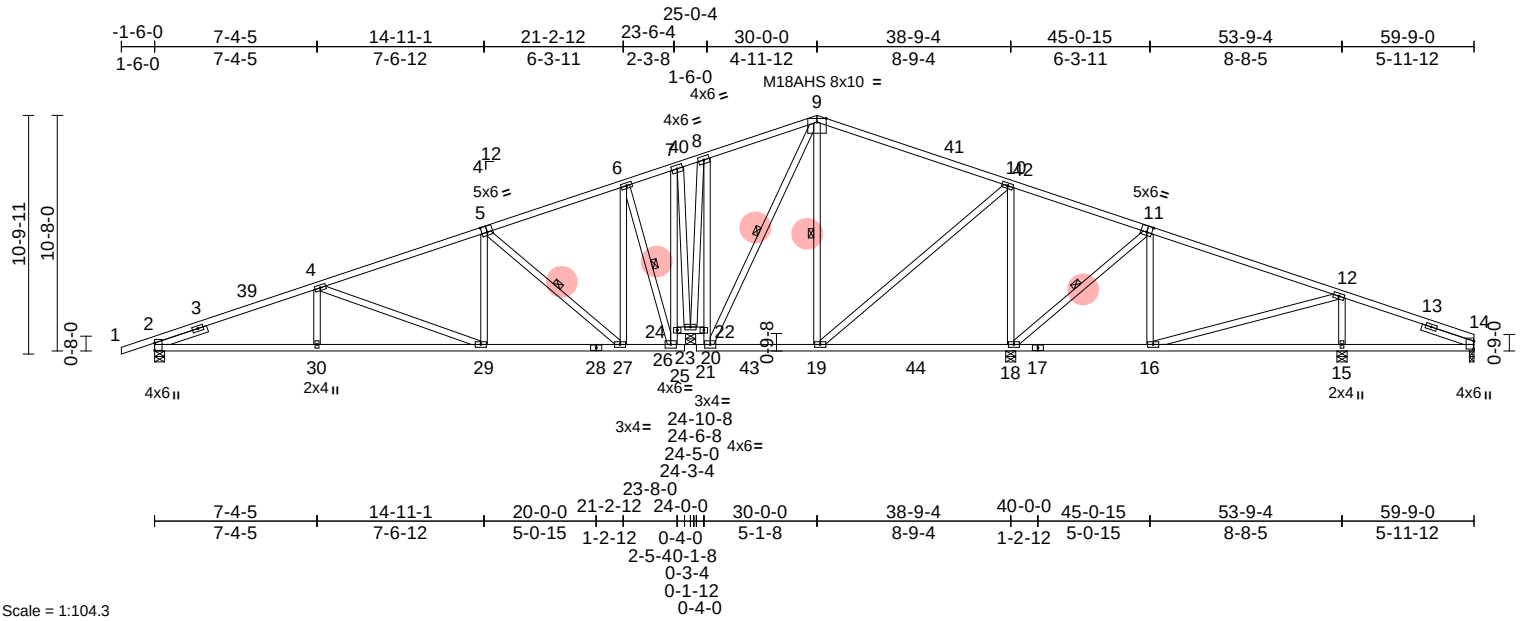

Scale = 1:104.3

Plate Offsets (X, Y): [2:0-4-1,0-0-1], [5:0-3-0,0-3-4], [11:0-2-12,0-3-4], [14:0-4-5,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.82	Vert(LL)	-0.14	18-19	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.75	Vert(CT)	-0.24	18-19	>738	180	M18AHS	169/162
TCDL	7.0	Rep Stress Incr	YES	WB	1.00	Horz(CT)	0.01	14	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 356 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr *Except* 26-7,8-20:2x4 DF Std
WEBS 2x4 DF Std
SLIDER Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied. Except: 10-0-0 oc bracing: 24-26, 20-22
WEBS 1 Row at midpt 9-19, 11-18, 6-26, 5-27, 9-20

REACTIONS (size) 2=0-5-8, 14=0-2-8, 15=0-5-8, 18=0-5-8, 23=0-5-8
Max Horiz 2=150 (LC 14)
Max Uplift 2=-161 (LC 10), 14=-37 (LC 15), 15=-119 (LC 15), 18=-151 (LC 11), 23=-205 (LC 14)
Max Grav 2=1030 (LC 3), 14=229 (LC 1), 15=837 (LC 4), 18=1618 (LC 4), 23=2060 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-4=-1867/269, 4-6=-1028/213, 6-7=0/333, 7-8=0/409, 8-9=-10/388, 9-10=-305/182, 10-12=-448/313, 12-14=-232/74
BOT CHORD 2-30=-281/1736, 29-30=-281/1736, 27-29=-130/908, 26-27=0/130, 25-26=0/0, 24-26=-156/1021, 7-24=-127/577, 23-24=-239/97, 22-23=-256/103, 20-22=0/643, 8-22=0/349, 20-21=0/0, 19-20=-51/144, 18-19=-211/74, 16-18=-17/334, 15-16=-34/153, 14-15=-47/153
WEBS 9-19=-30/233, 11-16=0/270, 12-15=-654/213, 12-16=0/189, 10-18=-1020/182, 11-18=-636/115, 4-30=0/283, 5-29=0/568, 4-29=-874/162, 6-27=-70/899, 6-26=-1081/182, 10-19=0/360, 5-27=-1057/179, 9-20=-643/10, 7-23=-590/122, 8-23=-635/102

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-0 to 4-5-11, Interior (1) 4-5-11 to 30-0-0, Exterior(2R) 30-0-0 to 35-11-11, Interior (1) 35-11-11 to 59-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 20.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 3x6 MT20 unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 14.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 14, 119 lb uplift at joint 15, 161 lb uplift at joint 2, 151 lb uplift at joint 18 and 205 lb uplift at joint 23.
11) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E6	Roof Special	2	1	R75436352
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:11 Page: 1
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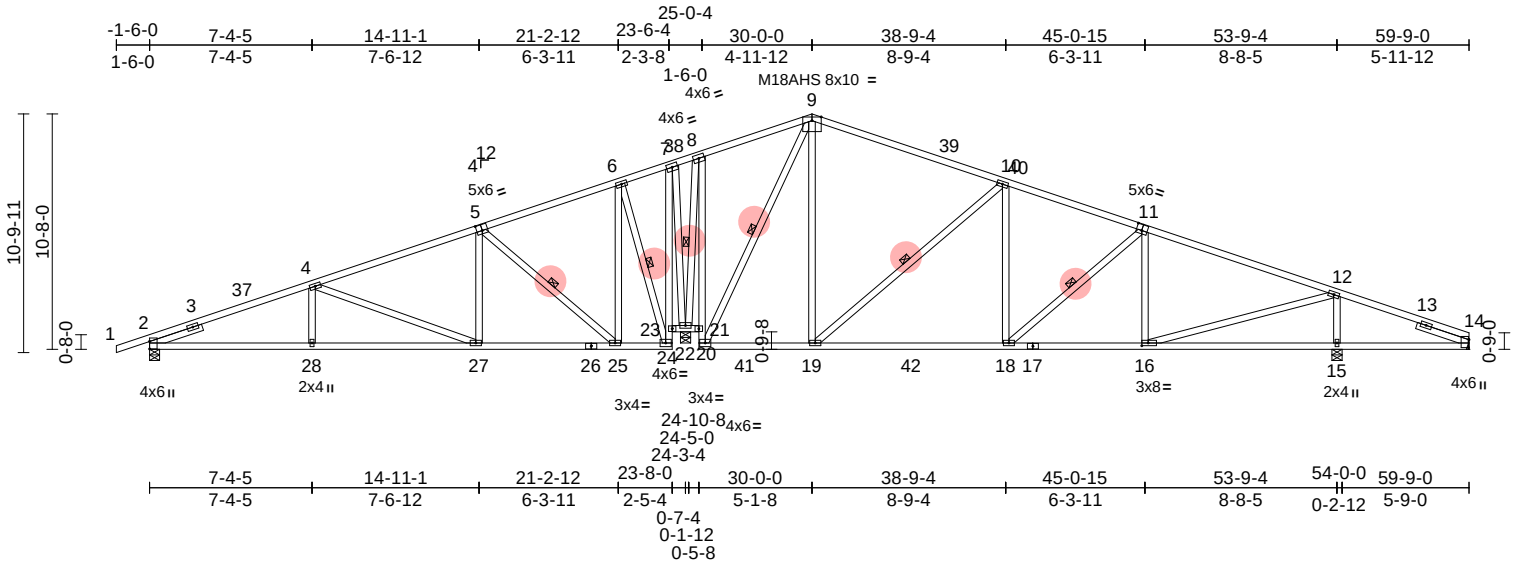


Scale = 1:104.3

Plate Offsets (X, Y): [2:0-4-1,0-0-1], [5:0-3-0,0-3-4], [11:0-3-0,0-3-4], [14:0-4-5,0-0-5], [16:0-3-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.87	Vert(LL)	-0.19	18-19	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.97	Vert(CT)	-0.32	18-19	>999	180	M18AHS	169/162
TCDL	7.0	Rep Stress Incr	YES	WB	1.00	Horz(CT)	0.01	14	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 355 lb	FT = 20%

LUMBER
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr *Except* 24-7,8-20:2x4 DF Std
WEBS 2x4 DF Std
SLIDER Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 11-18, 10-19, 5-25, 6-24, 9-20, 8-22

REACTIONS (size) 2=0-5-8, 14= Mechanical, 15=0-5-8, 22=0-5-8
Max Horiz 2=150 (LC 14)
Max Uplift 2=-162 (LC 10), 14=-29 (LC 15), 15=-200 (LC 15), 22=-217 (LC 10)
Max Grav 2=1015 (LC 3), 14=165 (LC 1), 15=1708 (LC 4), 22=2607 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-4=-1829/266, 4-6=-985/209, 6-7=0/385, 7-8=0/437, 8-9=-16/383, 9-10=-706/223, 10-12=-1659/281, 12-14=-195/102
BOT CHORD 2-28=-285/1700, 27-28=-285/1700, 25-27=-135/868, 24-25=-8/82, 23-24=-162/997, 7-23=-132/483, 22-23=-286/100, 21-22=-265/101, 20-21=-63/1451, 8-21=-43/884, 19-20=0/534, 18-19=-76/1387, 16-18=-129/1504, 15-16=-37/11, 14-15=-42/56
WEBS 9-19=-26/1072, 11-16=-278/124, 10-18=0/483, 11-18=-275/74, 4-28=0/284, 5-27=0/569, 4-27=-880/161, 6-25=-68/903, 10-19=-1119/183, 5-25=-1058/179, 12-15=-1472/293, 12-16=-127/1592, 6-24=-1074/180, 9-20=-1560/94, 7-22=-512/115, 8-22=-937/148

NOTES
1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-0 to 4-5-11, Interior (1) 4-5-11 to 30-0-0, Exterior(2R) 30-0-0 to 35-11-11, Interior (1) 35-11-11 to 59-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 20.0 psf or 1.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 3x6 MT20 unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 14, 200 lb uplift at joint 15, 217 lb uplift at joint 22 and 162 lb uplift at joint 2.
11) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

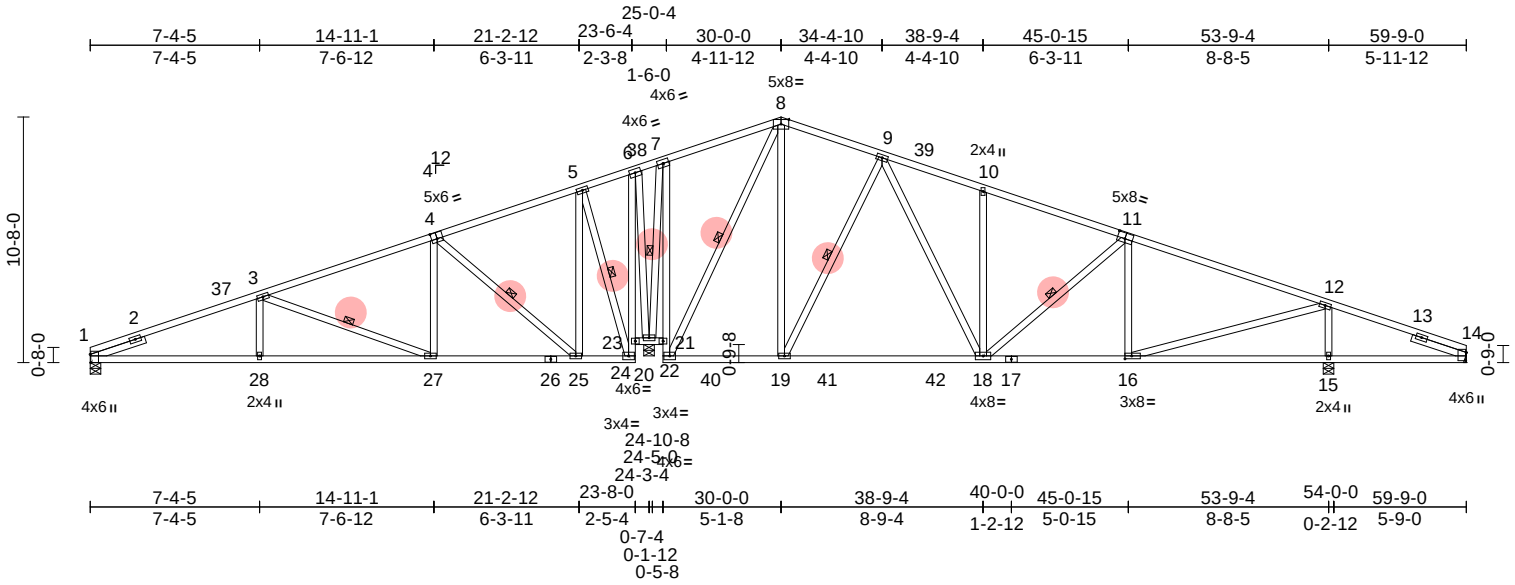
RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E7	Roof Special	1	1	R75436353
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:11 ID:noG129?QgKjpKcfaS2U?IszWPPE-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:100

Plate Offsets (X, Y): [1:0-4-5,Edge], [4:0-3-0,0-3-4], [11:0-4-0,0-3-0], [14:0-4-5,0-0-5], [16:0-3-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.55	Vert(LL)	-0.21	18-19	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.35	18-19	>999	180		
TCDL	7.0	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.02	14	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-MS								
BCDL	10.0										Weight: 364 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr *Except* 24-6,7-20:2x4 DF Std
WEBS	2x4 DF Std
SLIDER	Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-3-10 oc purlins.	
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.	
WEBS	1 Row at midpt	11-18, 3-27, 9-19, 4-25, 5-24, 8-20, 7-22

REACTIONS (size) 1=0-5-8, 14= Mechanical, 15=0-5-8, 22=0-5-8
Max Horiz 1=137 (LC 14)
Max Uplift 1=-114 (LC 10), 14=-29 (LC 15), 15=-202 (LC 15), 22=-213 (LC 10)
Max Grav 1=942 (LC 3), 14=175 (LC 1), 15=1683 (LC 4), 22=2610 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-3=-1866/300, 3-5=-1008/226, 5-6=0/369, 6-7=0/431, 7-8=-33/358, 8-9=-612/241, 9-10=-1460/327, 10-12=-1668/276, 12-14=-196/74

BOT CHORD 1-28=-298/1737, 27-28=-298/1737, 25-27=-143/889, 24-25=-7/96, 23-24=-165/1006, 6-23=-136/500, 22-23=-267/89, 21-22=-238/87, 20-21=-33/1386, 7-21=-14/821, 19-20=0/520, 18-19=-29/956, 16-18=-136/1517, 15-16=-21/29, 14-15=-46/72

WEBS 8-19=-86/1259, 11-16=-277/127, 10-18=-380/123, 11-18=-334/97, 3-28=0/284, 4-27=0/575, 3-27=-898/166, 5-25=-66/895, 9-19=-1041/199, 4-25=-1060/180, 12-15=-1444/294, 12-16=-136/1580, 5-24=-1075/181, 8-20=-1492/59, 6-22=-488/102, 7-22=-981/165, 9-18=-107/856

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-11-11, Interior (1) 5-11-11 to 30-0-0, Exterior(2R) 30-0-0 to 35-11-11, Interior (1) 35-11-11 to 59-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 14, 114 lb uplift at joint 1, 202 lb uplift at joint 15 and 213 lb uplift at joint 22.
9) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E8	Common	6	1	R75436354
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035,

Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:12 Page: 1
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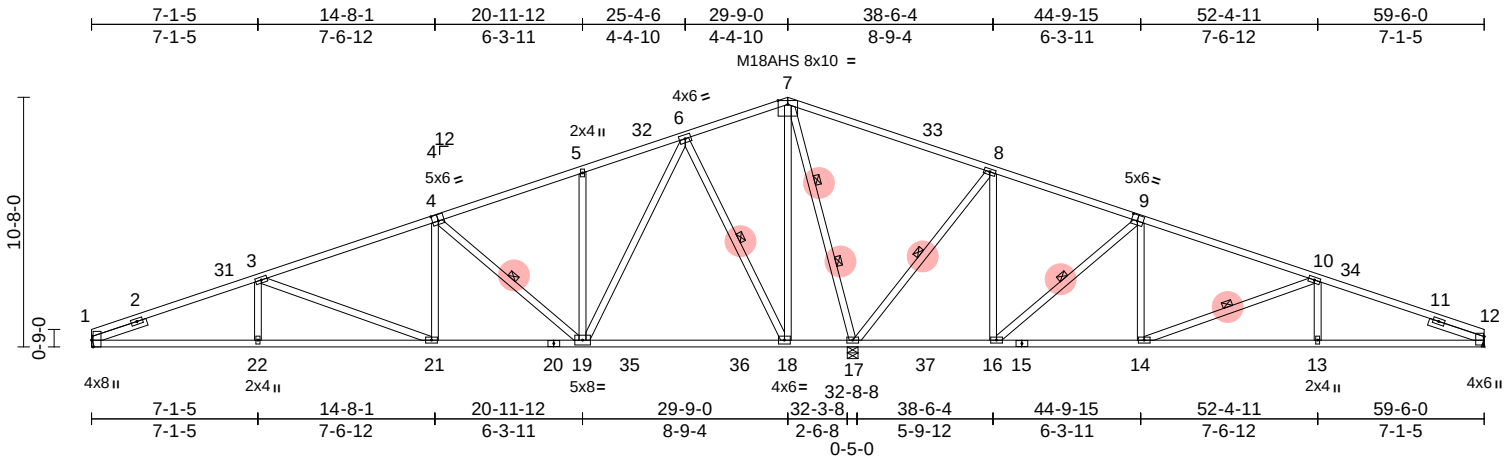


Scale = 1:98.4

Plate Offsets (X, Y): [1:0-3-8,Edge], [4:0-3-0,0-3-4], [9:0-3-0,0-3-0], [12:0-4-5,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	-0.24	18-19	>999	240	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.38	18-19	>999	180	M18AHS	169/162
TCDL	7.0	Rep Stress Incr	YES	WB	0.94	Horz(CT)	0.04	17	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-MS								
BCDL	10.0										Weight: 321 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr *Except* 4-7,9-7:2x4 DF 2400F 2.0E
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std *Except* 17-7:2x4 DF No.1&Btr
SLIDER	Left 2x4 DF Std -- 2-6-0, Right 2x4 DF Std -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-2-13 oc purlins.	
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	
WEBS	1 Row at midpt	10-14, 9-16, 6-18, 8-17, 4-19
WEBS	2 Rows at 1/3 pts	7-17

REACTIONS (size) 1= Mechanical, 12= Mechanical, 17=0-5-8
Max Horiz 1=-134 (LC 15)
Max Uplift 1=-134 (LC 14), 12=-125 (LC 15), 17=-240 (LC 10)
Max Grav 1=1052 (LC 20), 12=716 (LC 21), 17=3719 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-3=-2128/291, 3-5=-1408/228, 5-6=-613/213, 6-7=0/1061, 7-8=-55/1818, 8-10=-422/986, 10-12=-1195/270
BOT CHORD	1-22=-362/1973, 21-22=-362/1973, 19-21=-228/1259, 18-19=-517/207, 17-18=-966/229, 16-17=-908/161, 14-16=-449/314, 13-14=-209/1174, 12-13=-209/1174
WEBS	7-18=-123/1509, 9-14=0/588, 10-13=0/297, 10-14=-975/171, 8-16=-26/900, 9-16=-1007/156, 3-22=0/281, 4-21=0/517, 3-21=-831/148, 5-19=-391/133, 6-18=-1415/235, 8-17=-1483/246, 4-19=-1003/185, 7-17=-2635/277, 6-19=-179/1425

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-3-0 to 6-2-6, Interior (1) 6-2-6 to 30-0-0, Exterior(2R) 30-0-0 to 35-11-6, Interior (1) 35-11-6 to 59-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
3) Unbalanced snow loads have been considered for this design.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) Refer to girder(s) for truss to truss connections.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 1, 125 lb uplift at joint 12 and 240 lb uplift at joint 17.
10) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

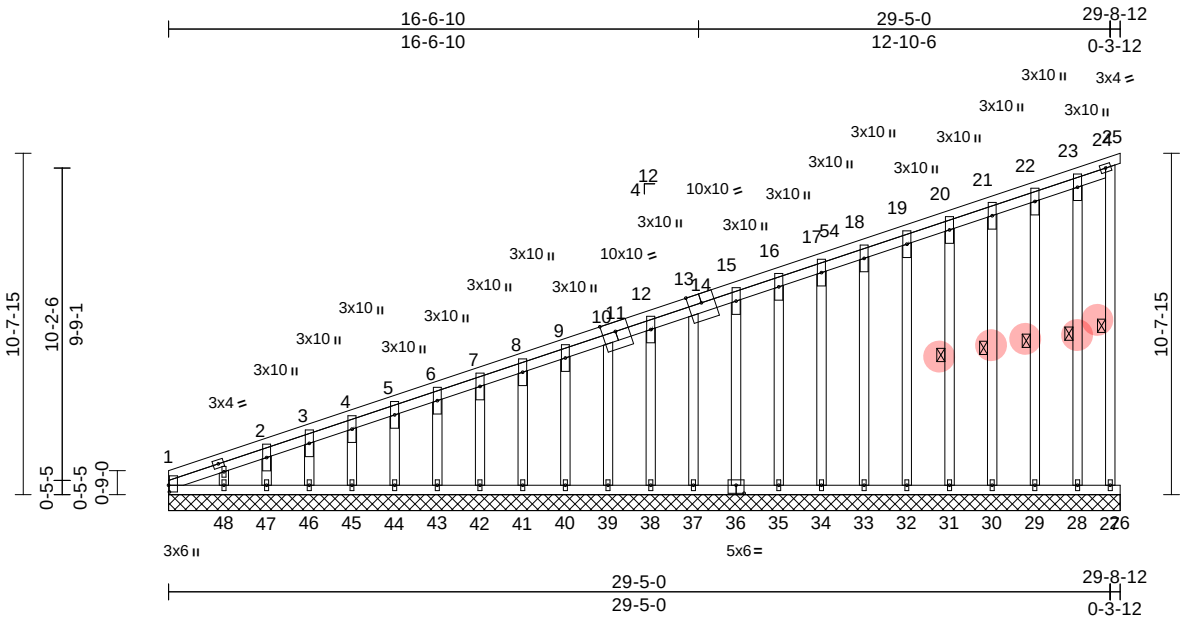
RENEWAL DATE: 12-31-2023
March 28,2023

MiTek USA, Inc.
400 Sunrise Avenue, Suite 270
Roseville, CA 95661

Job	Truss	Truss Type	Qty	Ply	J-3150
J-3150-1	E9GE	Monopitch Supported Gable	2	1	R75436355
					Job Reference (optional)

Pacific Lumber & Truss Co., Lake Oswego, OR - 97035, Run: 8.63 S Nov 19 2022 Print: 8.630 S Nov 19 2022 MiTek Industries, Inc. Tue Mar 28 13:20:12 Page: 1

ID:IuFd3TEK6StCQoOFSMNP92zWOtM-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:72

Plate Offsets (X, Y): [1:0-2-8,0-0-5], [36:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	220/195
(Roof Snow = 25.0)		Lumber DOL	1.15	BC	0.05	Vert(TL)	n/a	-	n/a	999		
TCDL	7.0	Rep Stress Incr	YES	WB	0.15	Horiz(TL)	-0.01	25	n/a	n/a		
BCLL	0.0*	Code	IBC2018/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 278 lb	FT = 20%

LUMBER

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 DF Std
OTHERS	2x4 DF Std

BRACING

TOP CHORD	Structural wood sheathing directly applied.	
BOT CHORD	Rigid ceiling directly applied.	
WEBS	1 Row at midpt	24-27, 23-28, 22-29, 21-30, 20-31

REACTIONS (size) 1=29-8-12, 25=29-8-12, 26=29-8-12, 27=29-8-12, 28=29-8-12, 29=29-8-12, 30=29-8-12, 31=29-8-12, 32=29-8-12, 33=29-8-12, 34=29-8-12, 35=29-8-12, 36=29-8-12, 37=29-8-12, 38=29-8-12, 39=29-8-12, 40=29-8-12, 41=29-8-12, 42=29-8-12, 43=29-8-12, 44=29-8-12, 45=29-8-12, 46=29-8-12, 47=29-8-12, 48=29-8-12, 50=29-8-12

Max Horiz 1=310 (LC 10), 50=310 (LC 10)

Max Uplift 25=-14 (LC 10), 26=-1 (LC 5), 28=-14 (LC 10), 29=-20 (LC 10), 30=-20 (LC 10), 31=-19 (LC 10), 32=-19 (LC 10), 33=-19 (LC 10), 34=-19 (LC 10), 35=-19 (LC 10), 36=-19 (LC 10), 37=-19 (LC 10), 38=-19 (LC 10), 39=-19 (LC 10), 40=-19 (LC 10), 41=-19 (LC 10), 42=-19 (LC 10), 43=-19 (LC 10), 44=-21 (LC 10), 45=-17 (LC 10), 47=-64 (LC 10), 48=-61 (LC 10)

Max Grav 1=151 (LC 10), 25=50 (LC 20), 26=0 (LC 10), 27=49 (LC 20), 28=136 (LC 20), 29=173 (LC 20), 30=171 (LC 20), 31=167 (LC 20), 32=166 (LC 20), 33=155 (LC 20), 34=127 (LC 20), 35=112 (LC 1), 36=112 (LC 1), 37=112 (LC 1), 38=112 (LC 20), 39=112 (LC 20), 40=112 (LC 1), 41=112 (LC 1), 42=112 (LC 1), 43=112 (LC 20), 44=113 (LC 20), 45=110 (LC 1), 46=91 (LC 1), 47=185 (LC 1), 48=70 (LC 5), 50=151 (LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-318/169, 2-3=-283/146, 3-4=-278/146, 4-5=-263/138, 5-6=-248/131, 6-7=-234/123, 7-8=-219/115, 8-9=-205/108, 9-10=-190/100, 10-12=-175/92, 12-13=-161/85, 13-15=-146/77, 15-16=-131/69, 16-17=-117/62, 17-18=-102/54, 18-19=-87/46, 19-20=-73/39, 20-21=-58/31, 21-22=-43/23, 22-23=-28/15, 23-24=-19/18, 24-25=-5/15

BOT CHORD 1-48=0/0, 47-48=0/0, 46-47=0/0, 45-46=0/0, 44-45=0/0, 43-44=0/0, 42-43=0/0, 41-42=0/0, 40-41=0/0, 39-40=0/0, 38-39=0/0, 37-38=0/0, 35-37=0/0, 34-35=0/0, 33-34=0/0, 32-33=0/0, 31-32=0/0, 30-31=0/0, 29-30=0/0, 28-29=0/0, 27-28=0/0, 26-27=0/0

WEBS 24-27=-34/9, 23-28=-111/28, 22-29=-146/37, 21-30=-144/36, 20-31=-141/35, 19-32=-139/35, 18-33=-128/35, 17-34=-100/35, 16-35=-85/35, 15-36=-85/35, 13-37=-85/35, 12-38=-85/35, 10-39=-85/35, 9-40=-85/35, 8-41=-85/35, 7-42=-85/35, 6-43=-85/35, 5-44=-86/36, 4-45=-84/35, 3-46=-61/3, 2-47=-171/106

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-3-0 to 3-3-12, Exterior(2N) 3-3-12 to 29-11-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=25.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; IBC 1607.11.2 minimum roof live load applied where required.
4) Unbalanced snow loads have been considered for this design.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 1-4-0 oc.

RENEWAL DATE: 12-31-2023

March 28,2023

Continued on page 2

MiTek USA, Inc.
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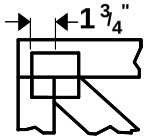
Roseville, CA 95661

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Bearing at joint(s) 25 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 25, 1 lb uplift at joint 26, 14 lb uplift at joint 28, 20 lb uplift at joint 29, 20 lb uplift at joint 30, 19 lb uplift at joint 31, 19 lb uplift at joint 32, 19 lb uplift at joint 33, 19 lb uplift at joint 34, 19 lb uplift at joint 35, 19 lb uplift at joint 36, 19 lb uplift at joint 37, 19 lb uplift at joint 38, 19 lb uplift at joint 39, 19 lb uplift at joint 40, 19 lb uplift at joint 41, 19 lb uplift at joint 42, 19 lb uplift at joint 43, 21 lb uplift at joint 44, 17 lb uplift at joint 45, 64 lb uplift at joint 47 and 61 lb uplift at joint 48.
12) This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

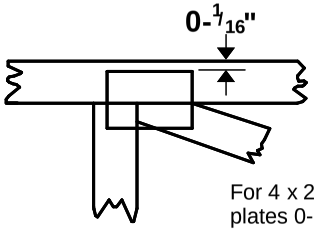
LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

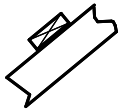
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

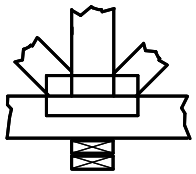
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

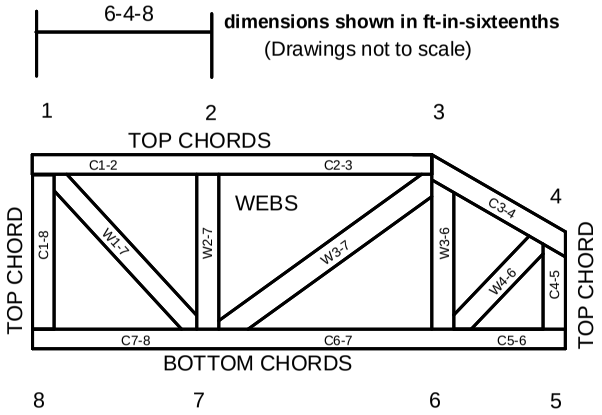


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

21.The design does not take into account any dynamic or other loads other than those expressly stated.

MICHAEL K. BROWN, S.E.
5346 E. Branchwood Dr., Boise, ID 83716
208-850-7542, mike.brown@core-engineering.net

LATERAL STRUCTURAL DESIGN CALCULATIONS

FOR

LOT 15, FARMSTEAD CROSSING
FOREST GROVE, OR

ENGR. JOB # DM23-038

Prepared for:
Stone Bridge Homes, NW, LLC
Plan #: 300
OTE: 3150
Date: 03-28-2023

The following calculations are for lateral wind and seismic engineering only. Gravity loading and the design of foundations are outside the scope of this design. The design is based on information provided by the client who is solely responsible for its accuracy. The engineering represents the finished product. Discrepancies from information provided by the client invalidate this design. Michael K. Brown, S.E. shall have no liability (expressed, or implied), with respect to the means and methods of construction workmanship or materials.

Michael K. Brown, S.E. shall have no obligation of liability, whether arising in contract (including warranty), Tort (including active, passive, or imputed negligence) or otherwise, for loss or use, revenue or profit, or for any other incidental or consequential damage.

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Job Name: Lot 15, Farmstead Crossing **By:** MKB

Core Job Number: DM23-038 **Date:** March, 2023

Code Basis of Design: Latest Adopted Version of O.S.S.C. and O.R.S.C.

Wind Loading: ASCE 7

Wind: 120 MPH ultimate wind speed, Exposure B, 4:12 Pitch roof front to back and Gable side to side Per ASCE 7-10 Figure 28.6-1:

Front To Back

Zone A: 31.6 PSF
Zone B: 0
Zone C: 21.1
Zone D: 0
For Zones A and B, a=10% of least horizontal dimension or 0.4h (h=mean roof height), but not less than 3'.
Least horizontal dimension = 40 feet
Therefore, use a = 4 feet

Side To Side

Zone A: 22.8 PSF
Zone B: 22.8
Zone C: 15.1
Zone D: 15.1
For Zones A and B, a=10% of least horizontal dimension or 0.4h (h=mean roof height), but not less than 3'.
Least horizontal dimension = 58 feet
Therefore, use a = 5.8 feet

Front to Back:

Average force on projected wall = (total force at zone A + total force at zone C)/overall length:
= 23.2 PSF
Average force on projected roof = (total force at zone B + total force at zone D)overall length:
= 0 PSF

Wall Plate Height = 9 feet
Projected Average Roof Height = 10 feet

Total Wind Force at Upper Level = 4176 #, or 104 PLF

Side to Side:

Horizontal Dimension = 58 Feet

Average force on projected wall = 16.6 PSF
Average force on projected roof = 16.64 PSF
Projected Average Roof height = 5 Feet
Total Wind Force at Upper Level = 9169 #, or 158 PLF

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Job Name: Lot 15, Farmstead Crossing **By:** MKB

Core Job Number: DM23-038 **Date:** March, 2023

Seismic Loading: D_1 seismic design category per O.R.S.C

S_{DS} = .83, R = 6.5, W = weight of structure
V = [1.2 S_{DS}/R] W
V = .154 W

Roof Dead Load =	17	psf
Floor Dead Load =	0	psf
Int. wall Dead Load =	6	psf
Ext. wall Dead Load =	12	psf

Seismic Front to Back:

Tributary Roof Length =	58	Feet
Tributary Floor Length =	0	Feet
Equiv. Trib Int wall Length =	20	Feet
Trib. Ext upper wall Height =	0	Feet
Trib. Ext lower wall Height =	4.5	Feet

Wroof = 178.64 plf > 104 plf, therefore seismic gov.

Total Seismic Load = 7145.6 Pounds > 4176 Therefore Siesmic Gov. overall

Seismic Side to Side:

Tributary Roof Length =	40	Feet
Tributary Floor Length =	0	Feet
Equiv. Trib Int wall Length =	20	Feet
Trib. Ext upper wall Height =	0	Feet
Trib. Ext lower wall Height =	4.5	Feet

Wroof = 131.516 plf < 158 plf, therefore wind gov

Total Seismic Force = 7627.928 Pounds, < 9169 Pounds, therefore wind gov. overall

Redundancy factor = 1.0 per ASCE 7, section 12.3.4.2

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Job Name: Lot 15, Farmstead Crossing **By:** MKB

Core Job Number: DM23-038 **Date:** March, 2023

Roof Level Wall Lines:

Line A:

P =	179	*	20 ft =	3573	#
Total Shear Wall Length =				17	Feet
V=P/L =	210	PLF			**Use A wall**

Wall Height =	9	Feet	Trib Roof Area =	2	Feet
Shortest Wall Length	17	Feet	Trib Floor Area =	0	Feet
Mot =	22509	LB FT	Factored Dead Load =	85	PLF
Mr =	12311	LB FT			
T =	600	LB			**No Uplift**

Line B:

P =	179	*	20 ft =	3573	#
Total Shear Wall Length =				24	Feet
V=P/L =	149	PLF			**Use A wall**

Wall Height =	9	Feet	Trib Roof Area =	2	Feet
Shortest Wall Length	24	Feet	Trib Floor Area =	0	Feet
Mot =	22509	LB FT	Factored Dead Load =	85	PLF
Mr =	24538	LB FT			
T =	-85	LB			**No Uplift**

Line 1:

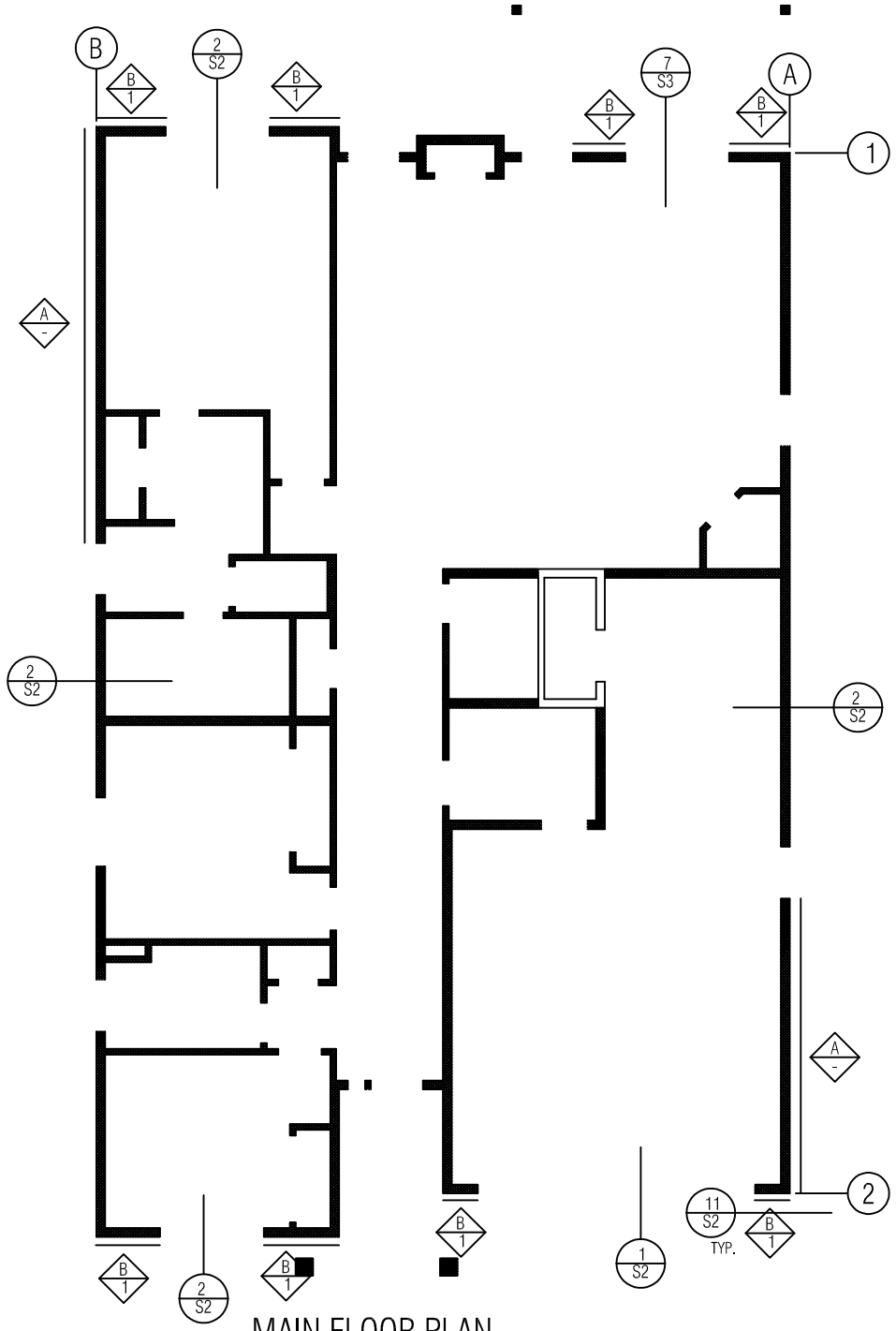
P =	158	*	29 ft =	4584	#
Total Shear Wall Length =				14	Feet
V=P/L =	327	PLF			**Use B wall**

Wall Height =	9	Feet	Trib Roof Area =	29	Feet
Shortest Wall Length	3	Feet	Trib Floor Area =	0	Feet
Mot =	6189	LB FT	Factored Dead Load =	361	PLF
Mr =	1623	LB FT			
T =	1522	LB			**Use HD type 1**

Line 2:

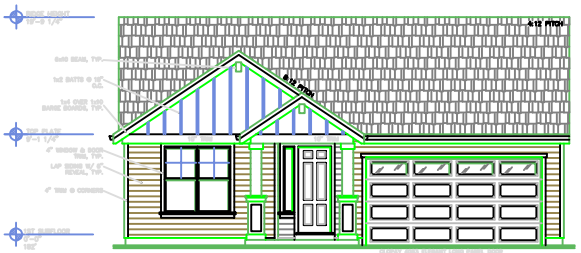
P =	158	*	29 ft =	4584	#
Total Shear Wall Length =				11	Feet
V=P/L =	417	PLF			**Use B wall**

Wall Height =	9	Feet	Trib Roof Area =	2	
Shortest Wall Length	2	Feet	Trib Floor Area =	0	
Mot =	5251	LB FT	Factored Dead Load =	85	PLF
Mr =	170	LB FT			
T =	2540	LB			**Use Type 1 HD**

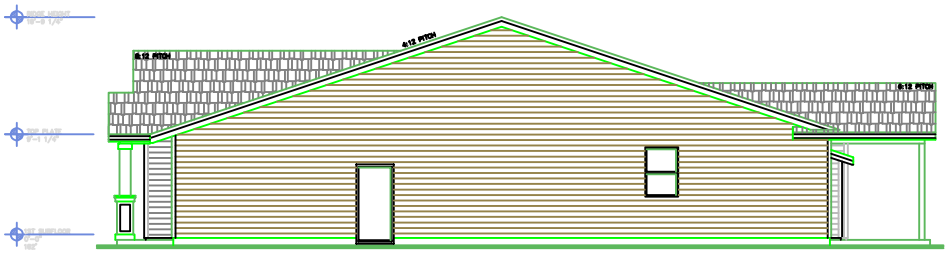


MAIN FLOOR PLAN

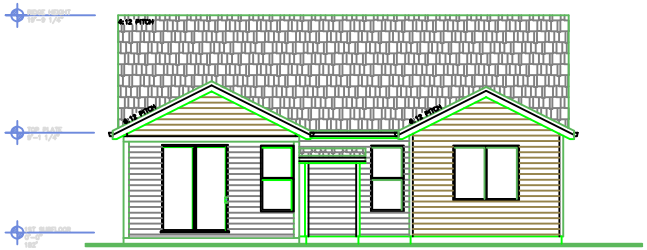
SCALE: 1/8"=1'



FRONT ELEVATION



RIGHT ELEVATION



REAR ELEVATION

LEFT ELEVATION

SHEARWALL SCHEDULE (a-m)

MARK NUMBER	REF NOTES: (a,i) SHEATHING	Note: (b) NAIL SIZE	EDGE NAILING SPACING	FEILD NAILING SPACING	SILL TO CONCRETE CONNECTION. Note: (c)	SILL TO WOOD CONNECTION. Note (g)	SHEAR TRNASFER CLIPS (h)	Lbs CAPACITY
A	7/16" OSB (1) SIDE	8d	6"	12"	1/2" Dia. A.B. @ 48" o/c	16d @ 6" o/c	A35 @ 24" o/c	280
B	7/16" OSB (1) SIDE (f)	8d	4"	12"	1/2" Dia. A.B. @ 24" o/c	16d @ 4" o/c	A35 @ 14" o/c	520
C	7/16" OSB (1) SIDE (e,f)	8d	3"	12"	1/2" Dia. A.B. @ 18" o/c (m)	16d @ 3" o/c	A35 @ 11" o/c	720
D	7/16" OSB (1) SIDE (e,f)	8d	2"	12"	1/2" Dia. A.B. @ 16" o/c (m)	16d @ 2" o/c	A35 @ 8" o/c	936
E	7/16" OSB (2) SIDE (d,e,f)	8d	6"	12"	1/2" Dia. A.B. @ 18" o/c (m)	16d @ 3" o/c	A35 @ 10" o/c	768
F	7/16" OSB (2) SIDE (d,e,f)	8d	4" Staggered	12"	1/2" Dia. A.B. @ 12" o/c (m)	16d @ 2" o/c	A35 @ 7" o/c	1120
G	7/16" OSB (2) SIDE (d,e,f)	8d	3" Staggered	12"	1/2" Dia. A.B. @ 10" o/c (m)	16d @ 3" o/c(2)rows staggered	A35 @ 5" o/c	1440
H	7/16" OSB (2) SIDE (d,e,f)	8d	2" Staggered	12"	1/2" Dia. A.B. @ 8" o/c (m)	16d @ 2" o/c(2)rows staggered	HGA10KT @ 7" o/c	1872

Notes:

a) Wood structural panels shall conform to the requireents for its type in DOC PS 1 or PS 2. All wall construction to conform to AF&PA SDPWS per IBC 2306.3.

b) Use Common Wire Nails for all wood sheathing and cooler nails for gypboard sheathing.

c) A.B. minimum 7" embed into concrete. 3"x3"x1/4" plate washers req'd at all shear wall A.B. Locate edge of Washer no greater than 1/2" from sheathed edge of sill plate.

d) Panel joints shall be offset to fall on different framing members or framing shall be 3x or thicker and nails on each side shall be staggered.

e) 3x or Dbl 2x framing at all panel edges and nails shall be staggered.

f) All edges blocked.

g) Common Wire Nails.

h) Clip to be attached from continuous blocking to top of continuous top plates.

Clips are not required at Gyp Bd walls but blocking is attached per the toenailing schedule.

i) See attached typical shearwall details.

j) Sheathing to be Structrual I Sheathing.

k) Values are for framing of H-F.

m) See plan for walls where seismic design shear is greater than 560 plf. 3x or 2x flat blkg at panel edges, Stagger nails.

Where indicated on the plans 3x sills are required with A.B. spacing as per plan and schedule. See note C for plate washers req'd at A.B.

For walls with the larger sills, Anchor Bolt spacing may be increased by a factor of 1.25 from the table above due to thicker sills.

HOLDOWN SCHEDULE

MARK NUMBER	HOLDOWN	Boundary Studs	Tension of DF Allowable Lbs	Tension of HF Allowable Lbs	Anchor Mono Pour	Anchor Two Pour
1	HDU2-SDS2.5 OR PHD2-SDS3	(2)2x	3075	2215	SSTB14	SSTB20L
2	HDU4-SDS2.5 OR PHD5-SDS3	(2)2x	4545	3270	SSTB16L	SSTB20L
3	HDU8-SDS2.5	(3)2x	7870	5665	SSTB28L	7/8"DIA.x24" MIN EMBED INTO CONC. A36 THREADED ROD w/ HEAVY SQ. NUT AND BP7/8-2 BEARING PLATE WASHER @ BOTT.
4	HDU11-SDS2.5	(1)6x	9535	6865	1"DIA.x42" A36 THREADED ROD w/ HEAVY SQ. NUT AND WASHER @ BOT. PROVIDE 24" EMBED INTO CONT FTG.	1"DIA.x42" A36 THREADED ROD w/ HEAVY SQ. NUT AND WASHER @ BOT. PROVIDE 24" EMBED INTO CONT FTG.
8	HHDQ14-SDS2.5	(1)6x	13710	10745		
5	MSTC28	(2)2x	3000	2590	N/A	N/A
6	MSTC40	(2)2x	4335	3745	N/A	N/A
7	MSTC66	(2)2x	5660	5660	N/A	N/A

Notes:

1. Install all holdowns per manufactureer specificaiton per latest Simpson Strong Tie catalog.
2. Match studs on schedule for walls below on all wall to wall holdowns.
3. (Multiple)2x studs nailed together with (2) rows of 16d @ 3" o.c. staggered.
4. Refer to shearwall schedule and typical shearwall details for wall locations and configurations.
5. Refer to Simpson catalog for minimum embed of anchors into concrete.

City of Forest Grove
1924 Council St
Forest Grove, OR 97116
503-992-3229
Fax: 503-992-3202

Residential

Certificate of Occupancy

Web Address: www.forestgrove-or.gov
Email Address: cd@forestgrove-or.gov

This structure has been inspected and approved according to the applicable codes, regulations and laws that were in effect at the time the permit was issued. All final inspections have been completed and this dwelling is approved for occupancy.

Residential Specialty Code Edition: 2021

Permit Number: 311-23-000206-DWL **Final Inspection Date:** 9/18/23

Property Address: 3730 TIANA ST, FOREST GROVE, OR 97116

Parcel Number: FARMSTEAD TEMPORARY

Owner: STONE BRIDGE HOMES NW LLC, 4230 Galewood Street, Suite #100, LAKE OSWEGO, OR 97035

Description of Work: NSFR- FARMSTEAD CROSSING - LOT 15 - PLAN 00 - 3 BED - 2 BATH - 1 FLOOR

Category of Construction: Single Family Dwelling **Type of Work:** New

Existing Sprinklers: Not specified **Sprinklers Included in Project:** Not specified

Portion of Building: NSFR

Special Conditions: None Specified

Occupancy Classification	Type of Construction	Sq. Ft.
R-3 1 & 2 family	VB	1,858
U Utility, misc.	VB	567
U Utility, misc. - half rate	VB	244

Yvette Hamilton
Building Official

Effective Date: September 19, 2023

Contact and license information for the general, electrical, plumbing and mechanical contractors is on file and can be obtained upon request.

Forest Grove Inspection Report

1924 Council Street
Forest Grove, OR 97116
Tel: (503)992-3229

Location:
3730 TIANA ST FOREST GROVE OR 97116

Inspection Date:
Mon, 18 Sep 2023 02:56 PM

Owner:
STONE BRIDGE HOMES NW LLC

Record ID:
311-23-000206-DWL

Record Type:
Residential 1 & 2 Fam Dwelling (New Only) Limited

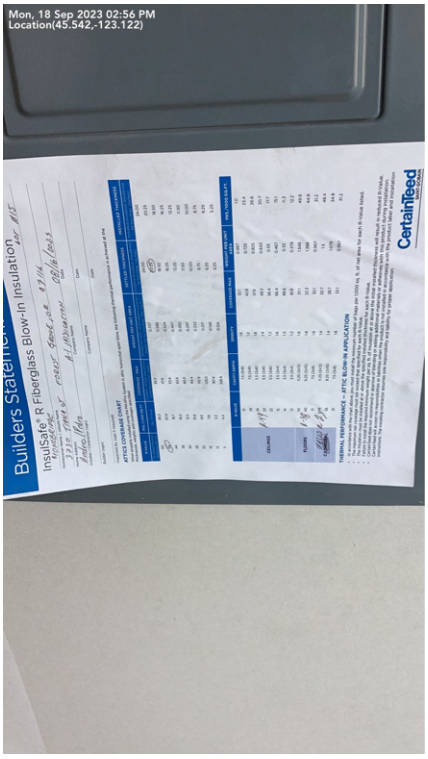
Result:
Approved

Inspection Type:
1999 Final Building

Inspector: Cassi Bergstrom
Phone: 503-992-3265
Email: cbergstrom@forestgrove-or.gov

Comments:
Address posted
Insulation certificate and blower door test received

Inspector


Mon, 18 Sep 2023 02:56 PM
Location(45.542,-123.122)
Builders Statement
InsulSafe R Fiberglass Blow-In Insulation
CertainTeed


Mon, 18 Sep 2023 02:56 PM
Location(45.542,-123.122)
earthadvantage.
HOME CERTIFICATION
PROOF OF CERTIFICATION
This label certifies this home was built in strict accordance with Earth Advantage green building guidelines and has passed two building inspections and rigorous performance testing.
3730 Tiona Street
ENERGY • HEALTH • LAND • MATERIALS • WATER
CertainTeed


Mon, 18 Sep 2023 02:56 PM
Location(45.542,-123.122)
CHARGER
240 VOLT
CAR CHARGER
AIR COND.
AIR COND.
SURGE PROTECTOR
SURGE PROTECTOR
PERFORMANCE TESTING RESULTS
HOMC30UC
SQUARE D

Forest Grove Inspection Report

1924 Council Street
Forest Grove, OR 97116
Tel: (503)992-3229

Location:
3730 TIANA ST FOREST GROVE OR 97116

Inspection Date:
Wed, 06 Sep 2023 01:52 PM

Owner:
LENNAR NORTHWEST INC

Record ID:
311-23-000607-PLM

Record Type:
Residential Plumbing

Result:
Approved

Inspection Type:
3620 Backflow Device

Inspector: Cassi Bergstrom
Phone: 503-992-3265
Email: cbergstrom@forestgrove-or.gov

Comments:
Located at meter. Model Febco 850; Serial No HG51685

Inspector